

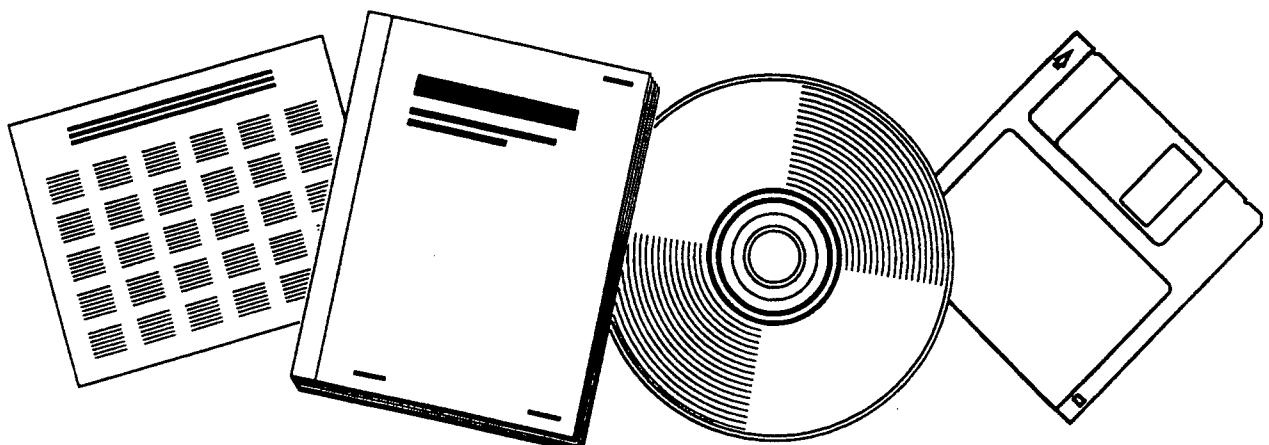


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TEMPERATURE ADJUSTMENT FACTORS FOR FALLING WEIGHT DEFLECTOMETER DEFLECTIONS ON FULL DEPTH ASPHALT CONCRETE PAVEMENTS

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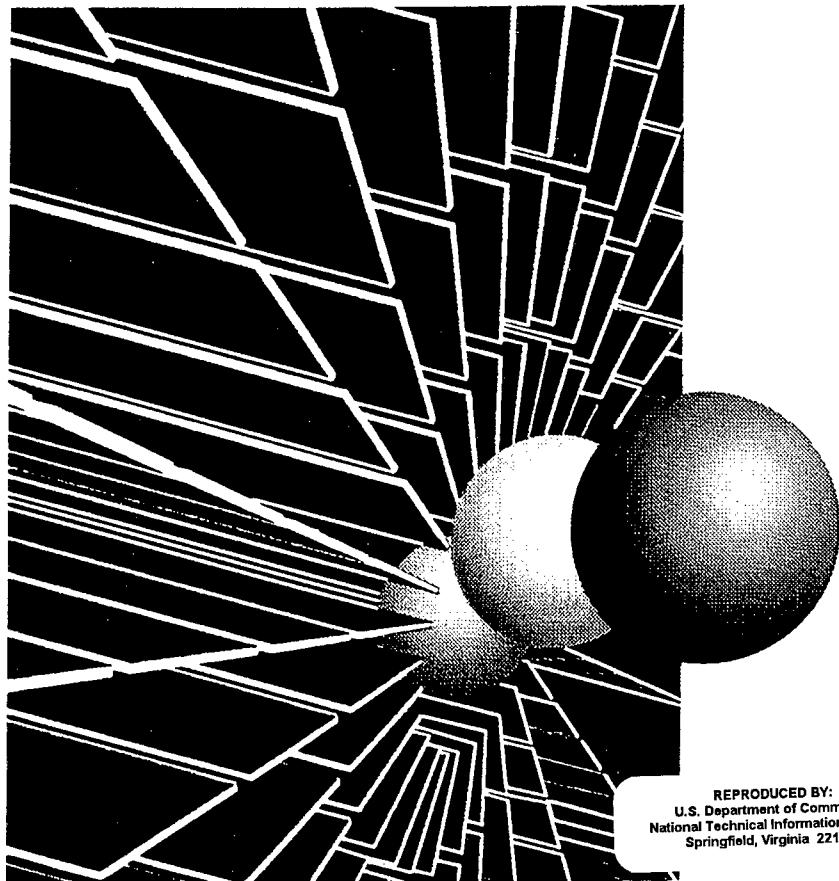
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Research, Development and Technology Division

RDT 97-002

Temperature Adjustment Factors
for Falling Weight Deflectometer
Deflections on Full Depth
Asphalt Concrete Pavements

Final Report



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16. Abstract A convenient way to gauge the structural capacity of a pavement is to compute the structural number (SN). To make the structural capacity of various pavements comparable to each other, the structural number, and therefore the Falling Weight Deflectometer (FWD) measured deflections that the SN is based on, must be corrected to a common temperature (68° Fahrenheit). In the following Research Investigation report, a Temperature Adjustment Factor curve (Temperature Adjustment Factor vs. Average Asphalt Concrete Mix Temperature) was developed in order to correct FWD deflections to 68°F. Also included in this report is the comparison of different methods used to determine the strength moduli of the pavement (E_p), asphalt concrete (E_{ac}), and the subgrade resilient modulus (M_r).			
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RESEARCH INVESTIGATION 91-9

**TEMPERATURE ADJUSTMENT FACTORS
FOR FALLING WEIGHT DEFLECTOMETER DEFLECTIONS
ON FULL DEPTH ASPHALT CONCRETE PAVEMENTS**

**PREPARED BY
MISSOURI DEPARTMENT OF TRANSPORTATION
RESEARCH, DEVELOPMENT AND TECHNOLOGY DIVISION**

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Acknowledgement to: MATT JOHNSON, Statistician

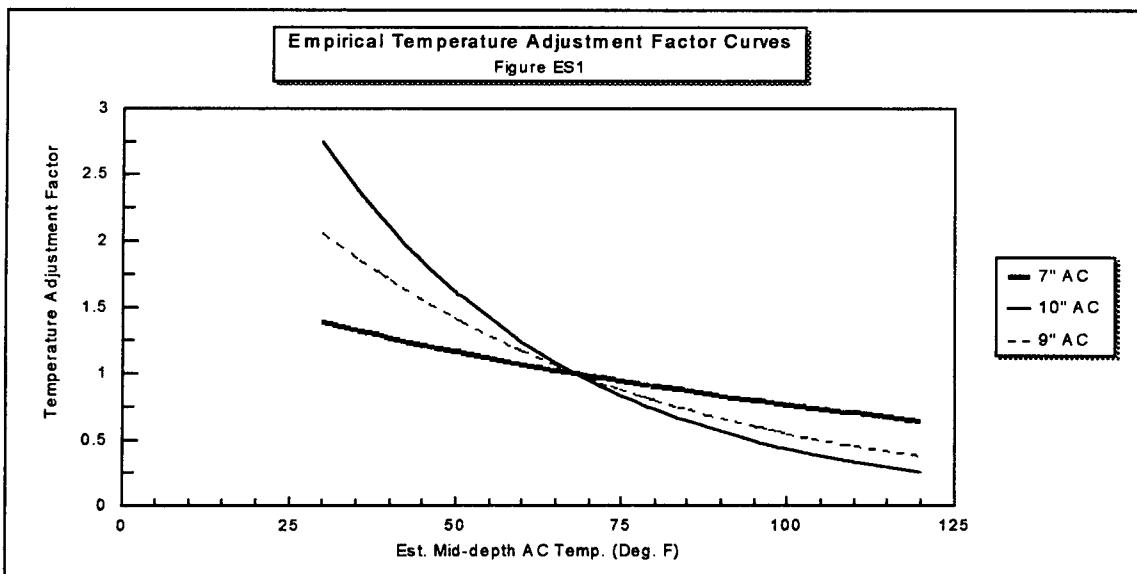
**JEFFERSON CITY, MISSOURI
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**The opinions, findings, and conclusions expressed in this publication are not necessarily
those of the Federal Highway Administration.**

EXECUTIVE SUMMARY

The Missouri Department of Transportation is currently considering ways to measure the structural capacity of a pavement through analyzing Falling Weight Deflectometer (FWD) deflection data. The structural capacity (or structural number) of various asphaltic concrete (AC) pavements can be compared only if the measured deflections are corrected to a standard temperature. The American Association of Highway Transportation Officials (AASHTO) recommends this temperature to be 68° F.

In Research Investigation (RI) 91-9, Temperature Adjustment Factor (TAF) curves (Temperature Adjustment Factor vs. Average Asphalt Concrete Mix Temperature) were developed based upon actual measured deflections and estimated mid-depth asphalt concrete temperatures. As a comparison, another set of Temperature Adjustment Factor curves were developed based upon theoretical deflections and assigned mid-depth AC temperatures. The empirical Temperature Adjustment Factor curves, which were developed by true deflections, are the curves recommended for MoDOT use on Falling Weight Deflectometer measurements. They can be found in Figure ES1 below.



With the above recommended TAF curve, a Falling Weight Deflectometer deflection value measured at T degrees can be multiplied by the temperature adjustment factor to get the equivalent deflection at 68° F. These equivocated deflections can then be used to calculate the structural number (SN) of a pavement which can be compared successfully to SN values of other pavements which have also been corrected to 68° F.

Also included in this Research Investigation report is a comparison of the methods used to find the strength moduli of a pavement (E_p), asphaltic concrete (E_{ac}), and the resilient modulus of the subgrade (M_r). No specific conclusions were made because laboratory data that would substantiate one method over the other was not available.

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LIST OF ABBREVIATIONS

a = non-destructive testing device load plate radius
a_e = effective radius of stress bulb induced in soil
AASHTO = American Association of State Highway and Transportation Officials
AC = asphalt concrete
AI = Asphalt Institute
d₀ = deflection measured at radius of 0 from applied load
D = total thickness of pavement layers above subgrade
D_r = deflection at distance r from applied load
E_{ac} = elastic modulus of asphalt concrete
E_b = strength modulus of base layer
E_p = elastic modulus of total pavement
F = loading frequency
FWD = Falling Weight Deflectometer
h_{ac} = height of asphalt concrete
h_b = height of base layer
h_{equivalent} = h_{Ep} = equivalent asphalt concrete height of base and original AC layer
MoDOT = Missouri Department of Transportation
M_r = resilient modulus of subgrade layer
MTxx = monthly testing site
 $n_{70 \text{ degrees}, 10^6}$ = absolute viscosity at 70 degrees F, in 10^6 poises
NDT = non-destructive testing
p = non-destructive testing device load plate pressure
P_{ac} = asphalt content, percent by weight of mix
P₂₀₀ = percent aggregate passing No. 200 sieve
RI = Research Investigation
RIMS = Regional Information Management System
SHRP = Strategic Highway Research Program
SN = structural number
SPS = Special Pavement Study
t_p = asphalt concrete mix temperature
T = temperature in degrees Fahrenheit
TAF = Temperature Adjustment Factor
 ν = Poisson's ratio
V_V = percent air voids
z = depth below pavement surface

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ABSTRACT

A convenient way to gauge the structural capacity of a pavement is to compute the structural number (SN). To make the structural capacity of various pavements comparable to each other, the structural number, and therefore the Falling Weight Deflectometer (FWD) measured deflections that the SN is based on, must be corrected to a common temperature (68° Fahrenheit).

In this Research Investigation report, a Temperature Adjustment Factor curve (Temperature Adjustment Factor vs. Average Asphalt Concrete Mix Temperature) was developed in order to correct FWD deflections to 68° F. Also included in the report is the comparison of different methods used to determine the strength moduli of the pavement (E_p), asphalt concrete (E_{ac}), and the subgrade resilient modulus (M_r).

OBJECTIVE

The objective of Research Investigation (RI) 91-9 is to adjust the deflections measured by the Falling Weight Deflectometer (FWD) on full-depth asphalt concrete pavements to a reference temperature of 68 degrees Fahrenheit as suggested by the American Association of State Highway and Transportation Officials (AASHTO). By establishing a Temperature Adjustment Factor curve (Temperature Adjustment Factor vs. Average Asphalt Concrete (AC) Mix Temperature), the structural number (SN) can be corrected to predict the structural capacity at 68 degrees Fahrenheit. Also, the correlation between load deflection and structural capacity of pavement will be improved using both classical theory and empirical relationships based on data analysis. (1)

There are several justifications for conducting an investigation to produce the aforementioned Temperature Adjustment Factor (TAF) curve for measured FWD deflections.

AASHTO states, "For purposes of comparison of E_p [pavement Elastic Modulus] along the length of a project, the d_0 [FWD deflection] values used to determine E_p should be adjusted to a single reference temperature." (2)

In the Strategic Highway Research Program (SHRP) Procedure for Temperature Correction of Maximum Deflections, it states, "Because of the temperature-dependent nature of the asphalt concrete modulus,..., measured deflections and hence structural capacity (or SN value) of the pavement will also vary with temperature. Thus, a procedure to correct the measured maximum deflection to a standard temperature is a valid one. Also, since the AASHTO structural number or SN value is computed at a standard temperature of 68 F, maximum deflection measured in the field must be corrected to this standard temperature." (3)

INTRODUCTION

In analyzing a Falling Weight Deflectometer deflection data set, the most straightforward interpretation of the information is to use the mean deflection value (normalized to a constant 9 kip equivalent load drop) for a given pavement. The mean pavement temperature for a given asphalt thickness should therefore be used in establishing a Temperature Adjustment Factor curve for correcting the FWD deflection data to a reference temperature. (In the context of our usage, "mean" is defined as the average throughout all depths of the AC pavement.) Ideally, to create an empirical Temperature Adjustment Factor curve, the true mid-depth temperature at the time of deflection data collection should be the most accurate mean temperature value. However, the exact mid-depth pavement temperature was not collected at each site as a part of this investigation, so another method of mid-depth temperature estimation had to be used.

In 1968, H.F. Southgate established a relationship between pavement surface temperature plus previous 5 day average air temperature and the temperature at a designated depth within asphalt pavement. (1) As a part of Research Investigation 91-9, Southgate's method was employed to create regression equations of our own, using MoDOT field temperature data from Monthly Testing Sites 1-9 (MT1-MT9) and Special Pavement Study 6 (SPS-6) test sites. Because of our now established correlation between surface temperature of AC plus previous 5 day average air temperature and temperature at a pavement depth., both the mean deflection and the estimated mid-depth pavement temperature were known for different MoDOT test sites. (For our purposes, we wanted to find the mean pavement temperature which was approximated as the mid-depth pavement temperature.) Thus, the proper parameters had been defined to create a Temperature Adjustment Factor curve using FWD deflection data. Of all the test sites, MT 1-3 were the only full depth AC pavements, so they became the focus from this point on in the investigation.

In order to establish a comparison to MoDOT's Temperature Adjustment Factor curve based on empirical data discussed above, another such curve was generated based on theoretical information. The deflection equation, derived by Boussinesq and Odemark presented in the 1993 AASHTO Design of Pavement Structures (2) was used to generate theoretical deflection data and create another Temperature Adjustment Factor curve. It is as follows:

$$d_0 = 1.5 p a \left\{ \frac{\frac{1}{M_R \sqrt{1 + \left(\frac{D}{a} \sqrt[3]{\frac{E_p}{M_R}} \right)^2}} + \left[1 - \frac{1}{\sqrt{1 + \left(\frac{D}{a} \right)^2}} \right]}{E_p} \right\}$$

The above equation will be discussed in depth in the following section, **INVESTIGATION PROCEDURE**, but some terms need some clarification at this point. Since the actual deflection data from MT1-MT3 (which was employed along with estimated mean AC mix temperatures to produce the empirical TAF curves) is a function of the subgrade resilient modulus and the pavement modulus of elasticity, these two components had to be varied in the theoretical procedure. To simulate the varying subgrade conditions due to seasonal changes and different site locations, the resilient modulus of the subgrade (M_r) required in the equation was evenly varied from 10,000 psi to 35,000 psi in 5000 psi increments to yield theoretical deflection data. However, the deflection information used in the TAF curve was averaged over all the M_r values to create the same methodology used in the empirical TAF, one with all of the various subgrades considered.

The effective pavement Modulus of Elasticity (E_p) used in the Odemark equation was a little more difficult to estimate since it depicts the material characteristics of the AC layer and the base layer combined. We calculated empirical E_{ac} (modulus of elasticity of asphalt concrete) values using the Asphalt Institute AC Elastic Modulus regression equation (2) using MoDOT typical mix properties and evenly varied AC mix temperature values as input variables. We did not, however, have access to a method that estimated a modulus strength value for the whole pavement structure, (E_p), versus varying temperatures. Therefore, the pavement modulus strength was approximated by using Odemark's transformed section equation to change the depth and strength of the base into an equivalent thickness of AC. This E_p is then equal to E_{ac} and the depth, D is the equivalent of h_{ac} . This procedure is discussed in depth in the following section,

INVESTIGATION PROCEDURE.

Finally, some comparisons were made between different methods used to find E_p , E_{ac} and M_r . By using the FWD deflection data, the MODULUS software program was run three ways: 1) as a 2 layer system (AC height plus base height considered one layer of pavement, and subgrade), 2) as a 3 layer system (AC pavement, base and subgrade) and 3) as a 4 layer system (AC pavement, base, first 36" of subgrade, and remaining subgrade). (MODULUS is a pavement layer strength backcalculation program that utilizes inputted FWD deflection data to estimate an appropriate deflection bowl for the data set.) The first MODULUS run yielded values for E_p and M_r which were compared to manually backcalculated solutions using the AASHTO guide section *Determination of Subgrade Modulus for S_{Neff} Determination* (2). The second and third runs both yielded values for E_{ac} but the third run had the least absolute error, therefore, it was compared to the Asphalt Institute AC Modulus regression equation results.

INVESTIGATION PROCEDURE

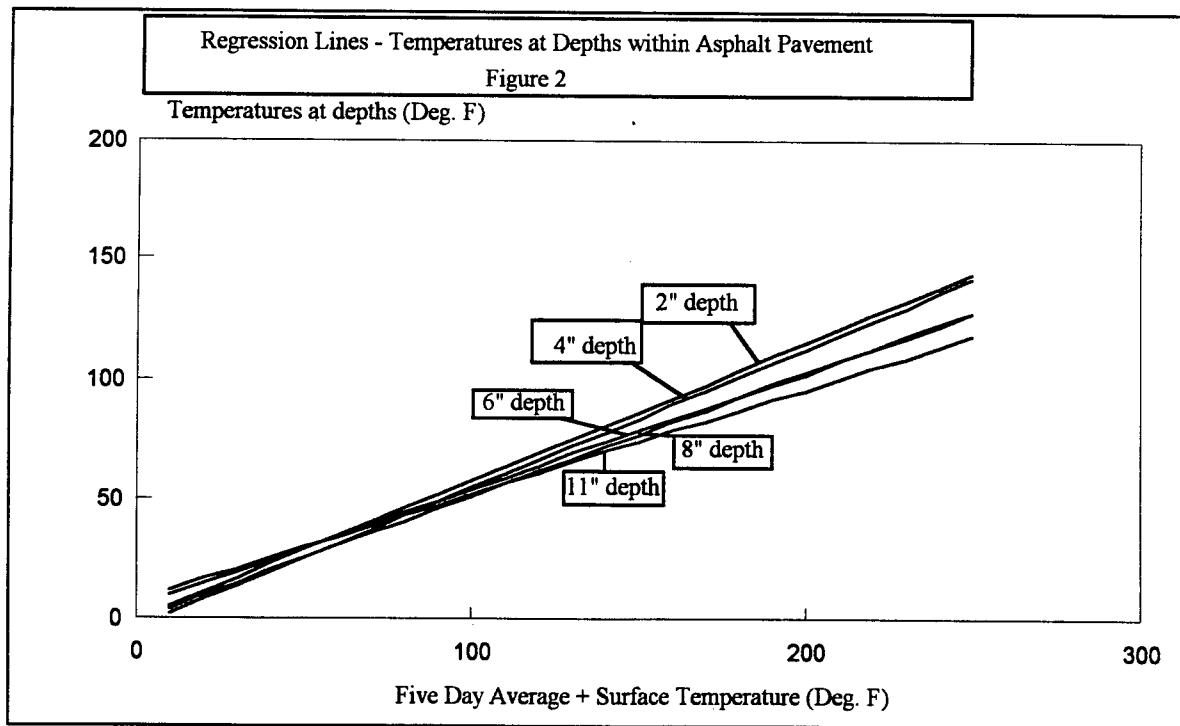
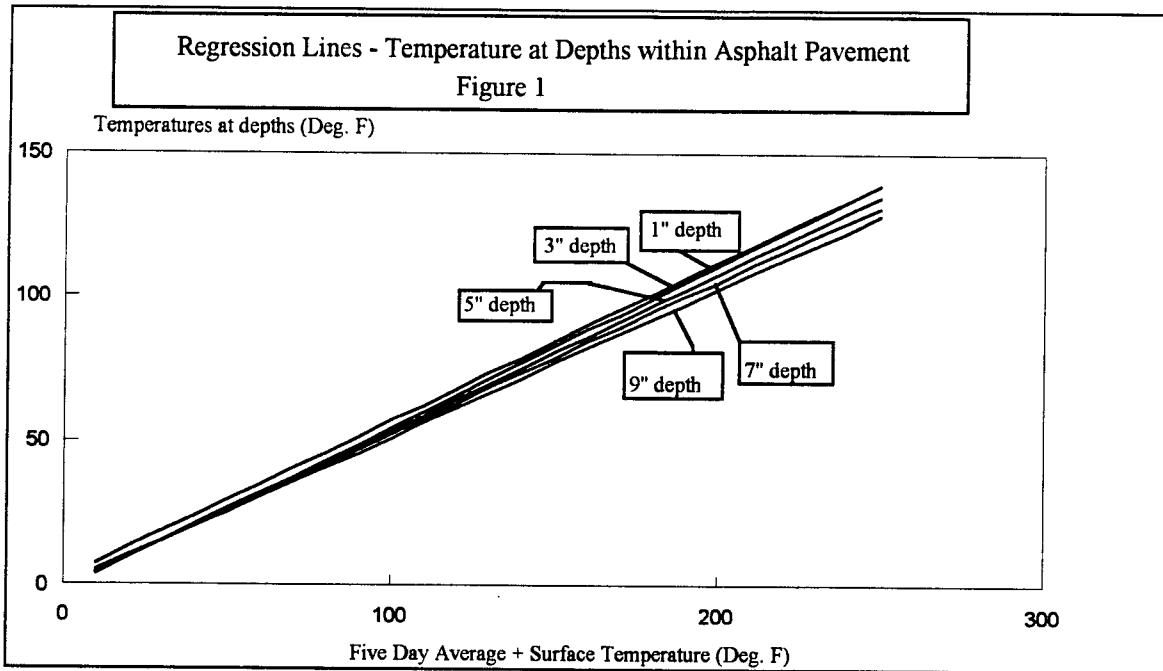
Empirical Temperature Adjustment Factor Curve

The first step in creating a Temperature Adjustment Factor curve is to establish a method to estimate the mid-depth or mean temperature of a given pavement. Field data was collected at nine monthly testing sites (MT1-MT9) as well as at SHRP site SPS-6. The data included: FWD deflection data sets, asphalt surface temperature, asphalt temperature at various (pre-determined) pavement depths, ambient air temperature at time of collection, and date of data collection (see Appendix A, pages A1-A19 for actual data). The high and low temperatures for the five days previous to data collection (for five day ambient air temperature average) were taken from National Weather Station data closest to each site.

As in Southgate's study (*1*), MoDOT wanted to verify a relationship between Five Day Ambient Air Temperature Average + Asphalt Surface Temperature versus Temperature At a Designated Depth. This way, if the asphalt surface temperature and five day average temperature were known at a site, one could use the association determined to estimate the temperature at pavement mid-depth without drilling holes and manually measuring the temperature. With this relationship, a significant savings of time, money and manpower could be realized if the FWD is used for inventory purposes. Regression equations were found for the 1, 2, 3, 4, 5, 6, 7, 8, 9, and 11 inch depth plots and can be seen in Figures 1 and 2 on the next page (see Appendix A, pages A20-A32 for a full page view of each graph as well as the calculated regression equations).

In this study, Southgate states, "...analysis of the standard error of estimate [of pavement depth temperatures] showed that the five-day average air-temperature history sufficed for all depths greater than 2 inches. The least standard errors of estimate for the depths 0 inches through 2 inches indicated that the best estimate was obtained by the use

of the surface temperature alone. Pavement temperatures in the top 2 inches of the pave



ment are directly dependent upon the hour of the day and the amount of heat absorption whereas, temperatures at depths greater than 2 inches are assumed to be a function of the surface temperature, amount of heat absorption, and the past five days of temperature history." (1) This information was very useful to us in analysis of the SPS-6 data.

For this site, the surface temperature readings were absent but the temperature was taken at the 1 inch depth and beyond. The SPS-6 data was essential to this portion of the investigation because it contained the majority of the various measured pavement depth temperatures, as can be seen in Appendix A. We felt that it couldn't be thrown out or disregarded in spite of the missing information. Instead, because of Southgate's conclusions previously discussed, the measured 1 inch depth temperature was used for the surface temperature reading as well as for the 1 inch depth temperature reading.

Southgate takes into consideration the different times of the day and had sufficient data to compose graphs showing pavement depth temperature relationships at certain hours of the day. This was another variance in our replication of his study because the data used in RI 91-9 was taken at different times of the day. Because of this fact, it is hard to directly compare the results of Southgate's study to the regression equations found in the relationships established by the RI 91-9 data. However, a general comparison can be made: all of the regression equations are fit to a linear relationship with a positive slope less than or very close to one.

Now, for each FWD deflection data set, the estimated mid-depth temperature of the pavement tested may be read off of the Five Day Air Temperature Average + Asphalt Surface Temperature versus Temperature at Pavement Depth graphs by using the appropriate mid-depth line. For example, if a section of pavement is 10 inches thick, the mid-depth temperature can be found on the 5 inch depth regression line. The temperature at 5 inches, or mid-depth will be read on the y-axis corresponding to the added temperature of the asphalt surface and the five day air average found on the x-axis.

In Table 1 below, the average measured FWD deflections and their estimated mid-depth temperatures can be found. The deflection measured directly underneath the Falling Weight Deflectometer load plate at a radius of zero is designated by d_0 .

TABLE 1: Estimated Mid-depth AC temperature and actual FWD d_0 deflection
 (at the time of testing). These FWD d_0 deflections include the varying subgrade M_r conditions and values.

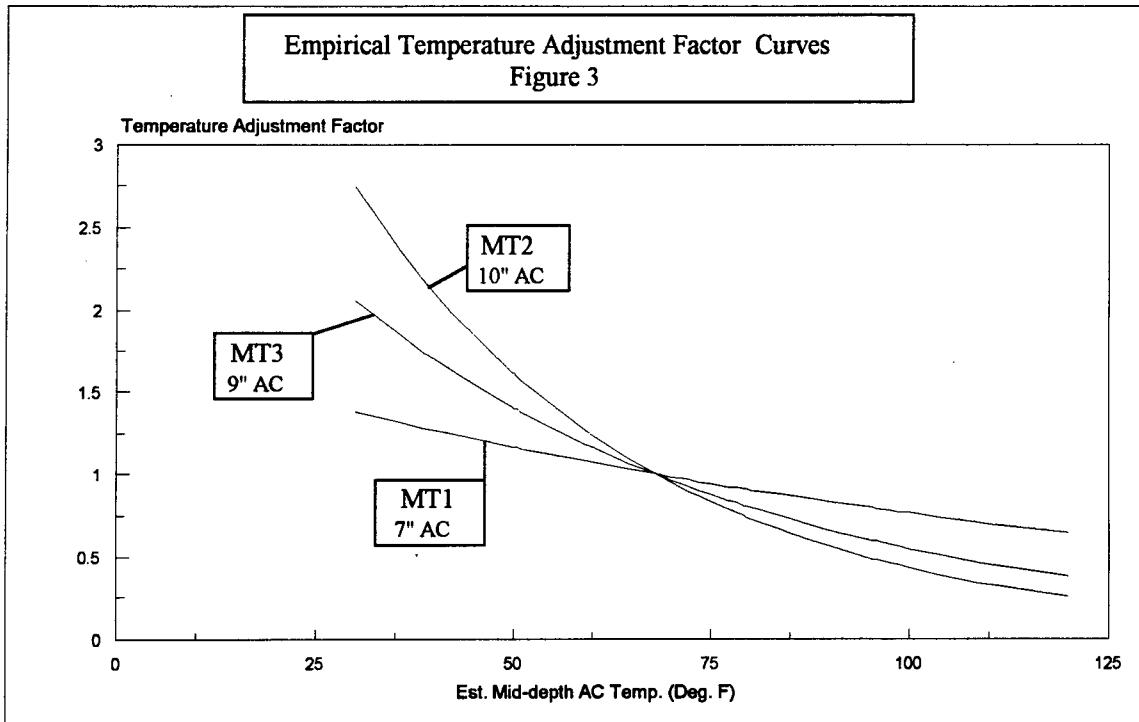
Site No.	Date Tested	5-day+surf. temperature (Deg. F)	Est. mid- depth temp. (Deg. F)	Measured	
				FWD d_0 (mils)	
MT1-1002	6-06-91	163	90.0	10.26	
MT1-1002	7-05-91	187	103.5	13.53	
MT1-1002	9-05-91	159	87.8	8.73	
MT1-1002	10-03-91	136	74.8	8.01	
MT1-1002	11-12-91	78	42.1	6.74	
MT1-1002	2-11-92	72	38.7	6.73	
MT1-1002	3-19-92	92	50.0	8	
MT1-1002	5-18-92	142	78.2	8.72	
MT2-1008	3-26-91	140	73.3	7.08	
MT2-1008	4-24-91	137	71.9	7.8	
MT2-1008	5-30-91	164	85.1	11.7	
MT2-1008	6-25-91	189	97.3	15.92	
MT2-1008	7-22-91	212	108.6	21.97	
MT2-1008	9-04-91	152	79.2	7.84	
MT2-1008	10-01-91	148	77.2	6.96	
MT2-1008	1-28-92	94	50.8	4.25	
MT2-1008	3-24-92	92	49.8	4.89	
MT2-1008	5-20-92	161	83.6	8.39	
MT3-606	6-04-91	181	95.3	23.68	
MT3-606	7-02-91	187	98.4	20.81	
MT3-606	7-30-91	182	95.8	19.4	
MT3-606	9-10-91	151	79.4	11.35	
MT3-606	10-09-91	112	58.8	7.77	
MT3-606	11-05-91	68	35.6	6.54	
MT3-606	1-30-92	88	46.1	7.87	
MT3-606	3-31-92	96	50.4	9.49	
MT3-606	5-27-92	123	64.6	11.64	

In order to find the Temperature Adjustment Factors from the above data, a best

fit exponential line was determined for each deflection versus estimated mid-depth temperature series (see Appendix B, pages B1-B4). Next, deflection values were predicted by these curve equations for temperatures at even intervals ranging from 30 degrees Fahrenheit to 120 degrees Fahrenheit (see Appendix B, page B5). Finally, the ratio of the deflection value at 68 degrees Fahrenheit to the deflection value at the varying temperatures produced the TAF curves found in Figure 3 below (for a full page view of Figure 3, see Appendix B, page B6). In summary,

$$TAF = \frac{d_{0_{68}}}{d_{0_T}}$$

where T is the temperature in degrees Fahrenheit that the deflection is being adjusted from (to 68 degrees Fahrenheit).



Theoretical Temperature Adjustment Factor Curve

To serve as a comparison to the empirical TAF curve above, a theoretical Temperature Adjustment Factor curve will now be developed.

As discussed earlier, deflection data was generated by using the Odemark approximate method for determining deflection in a two layer system. It is a combination of Boussinesq's one-layer equation and the concept of "equivalent thickness" described in 1940 by Barber. The deflection measured at the surface at the center of loading is assumed to be the sum of the subgrade and pavement deflections (2) and can be computed as follows:

$$d_0 = 1.5 p a \left\{ \frac{1}{M_R \sqrt{1 + \left(\frac{D}{a} \sqrt[3]{\frac{E_p}{M_R}} \right)^2}} + \frac{\left[1 - \frac{1}{\sqrt{1 + \left(\frac{D}{a} \right)^2}} \right]}{E_p} \right\}$$

where:
 d_0 = deflection measured at the center of the FWD load plate (inches)
 p = NDT load plate pressure (psi)
 a = NDT load plate radius (inches)
 D = total thickness of the pavement layers (AC and base layers) above the subgrade, (inches)
 M_R = subgrade resilient modulus (psi), and
 E_p = effective Modulus of all pavement layers above the subgrade (psi).

In order to get deflection values that are representative of the empirical data and TAF curve, several of the values in the Odemark-Boussinesq equation were varied. All of the generated information was then averaged by calculating a regression equation over all the theoretical TAF data points.

The first step was to vary the resilient modulus of the subgrade (M_R) from 10 ksi to 35 ksi, which were the estimated low and high limits for the subgrades tested.

As previously mentioned, estimating values for E_p was a little more involved. Because of the Asphalt Institute Elastic Modulus Regression equation as found in the

1993 AASHTO Design of Pavement Structures (2), temperature dependent values of E_{ac} were known. Average mix properties were inserted, where values were needed, yielding the equation below. Below is the Asphalt Institute Equation and a list of the mix properties that were used.

$$\log E_{ac} = 5.553833 + 0.028829 \left(\frac{P_{200}}{F^{0.17033}} \right) - 0.03476 V_v \\ + 0.070377 n_{70 \text{ deg rees } F, 10^6} + 0.000005 t_p^{(1.3+0.49825 \log F)} P_{ac}^{0.5} \\ + 0.931757 \left(\frac{1}{F^{0.02774}} \right)$$

where:

E_{ac} = elastic modulus of AC, psi (unknown)

P_{200} = percent aggregate passing the No. 200 sieve, (6%)

F = loading frequency, (18 Hz)

V_v = air voids, percent, (5%)

$n_{70 \text{ degrees}, 10^6}$ = absolute viscosity at 70 degrees F, 10^6 poise, (2)

P_{ac} = asphalt content, percent by weight of mix, (6%)

t_p = AC mix temperature, degrees F (varied)

After inserting these mix properties, the resulting equation is as follows:

$$\log|E| = 6.486476 - 1.8038865 * 10^{-4} * t_p^{1.92544}$$

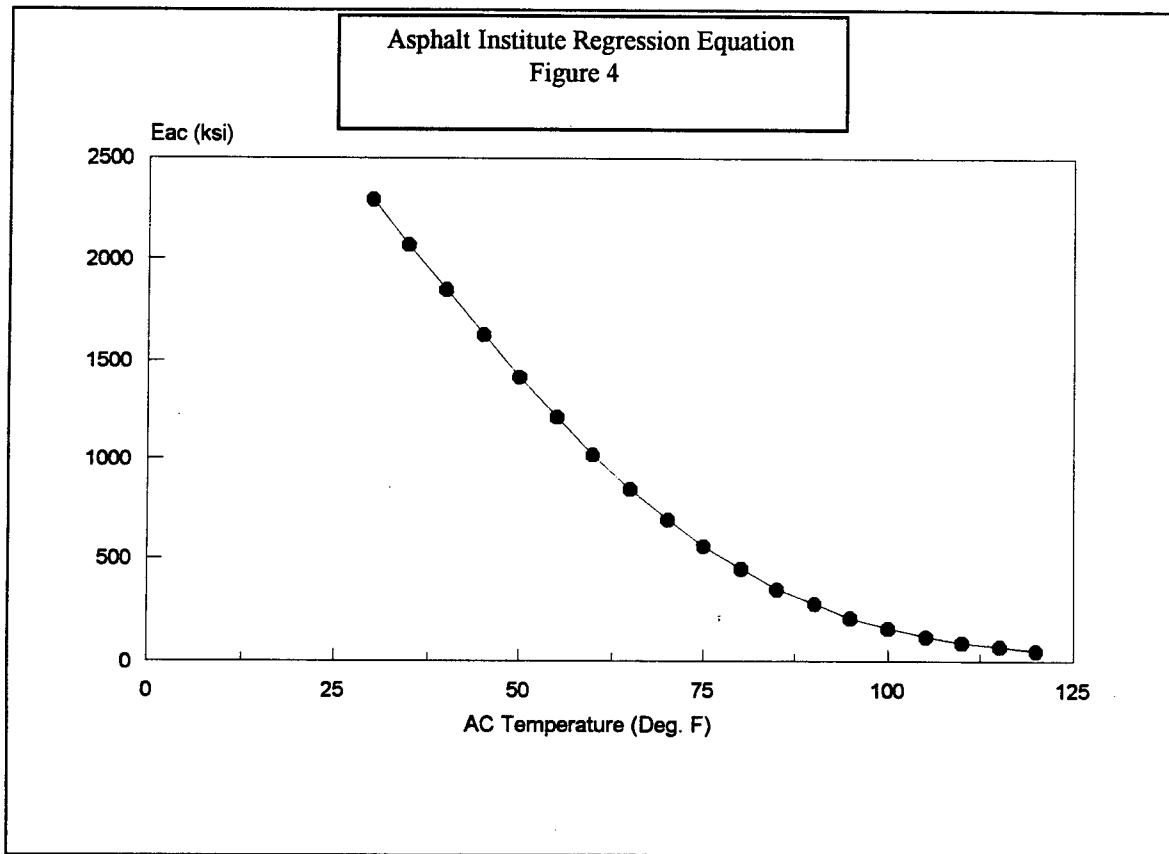
The calculated E_{ac} results from varying temperatures can be found in Table 2.

A graphical representation of the data from the resulting equation above can be found in Figure 4 on the next page. It can be easily seen from this graph that asphalt pavement strength is indeed temperature dependent.

Since the E_{ac} over the normal temperature range is now estimated, and the Odemark-Boussineq deflection equation requires E_p , a relationship between E_p and

TABLE 2: Estimated Mid-depth Temperature vs. Modulus of Elasticity for Asphalt Concrete

tp (deg. F)	Eac (psi)	tp (deg. F)	Eac (psi)
30	2.293E+06	75	5.637E+05
35	2.075E+06	80	4.506E+05
40	1.850E+06	85	3.554E+05
45	1.627E+06	90	2.766E+05
50	1.411E+06	95	2.124E+05
55	1.207E+06	100	1.610E+05
60	1.018E+06	105	1.204E+05
65	8.476E+05	110	8.893E+04
70	6.959E+05	115	6.482E+04
		120	4.663E+04



E_{ac} must be established. E_p is defined as the strength modulus of the overall pavement structure while E_{ac} is the strength modulus of the asphalt only.

The E_{ac} values found by the Asphalt Institute Regression Equation may be used as an estimate of E_p if the base is transformed into an equivalent height of asphalt. The following equation is used to determine an equivalent thickness of asphalt (e.g., transforming the base height to an AC height) whose strength is equal to the strength exhibited by E_{ac} and E_b (the strength modulus of the base) combined:

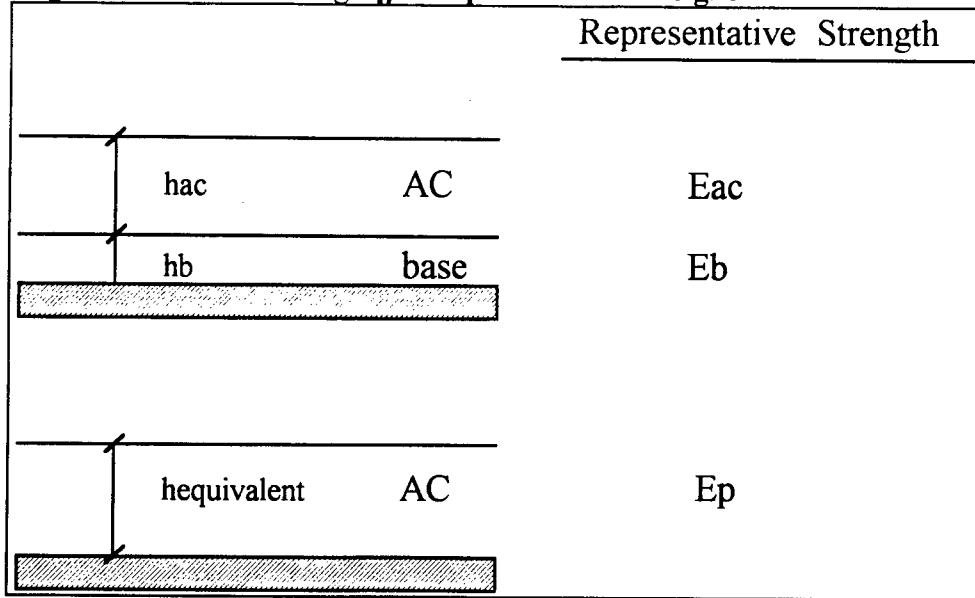
$$h_{equivalent} = h_b * \sqrt[3]{\frac{E_b}{E_{ac}}} + h_{ac}$$

The above equation is based on the concept of equivalent thickness as described in 1986 AASHTO Guide for Design of Pavement Structures (4). The design guide develops the interrelationship between layer coefficients and elastic moduli. In finding an equivalent thickness, the Poisson's ratio was assumed to be equal for both layers, and therefore, cancels out. (See sample calculations in Appendix C, pages C1 and C2).

The base heights for Monthly Testing sites 1-3 were all approximately 4 inches, therefore, in the above equivalent height equation, $h_b = 4$ inches. The average E_b found by the MODULUS program for the three testing sites was 60 ksi with a standard deviation of around 30 ksi. The three values of 30 ksi, 60 ksi and 90 ksi were used for E_b because of the high deviation value. The height of the asphalt pavement, h_{ac} , was modified in increments of 2 inches from 2 inches up to 12 inches in order to represent the spectrum of possible asphalt thicknesses in pavements. (See tabulated form of results in Appendix C, pages C3-C20.) Again, the deflection information generated by these values being varied was averaged by calculating a regression equation over all TAF data points produced.

The diagram in Figure 5 on the next page is an effort to make the equivalent height procedure more clear. Because the calculated AC equivalent height is representative of the strength of E_p at a particular temperature, it can be used in the Odemark-Boussinesq equation as "D", the total depth of the pavement, while the corresponding value of E_{ac} at that temperature is used in place of E_p .

Figure 5: Transforming h_b to Equivalent AC Height



The only values left in the Odemark equation that haven't yet been defined are p , the contact pressure, and a , the circular load radius. The contact pressure, equal to 82.3 psi, is the applied equivalent load of 9000 lbs (9 kips) divided by the area of the load plate. The circular load radius, also used to find the load plate area for the contact pressure computation, is equal to 5.9 inches.

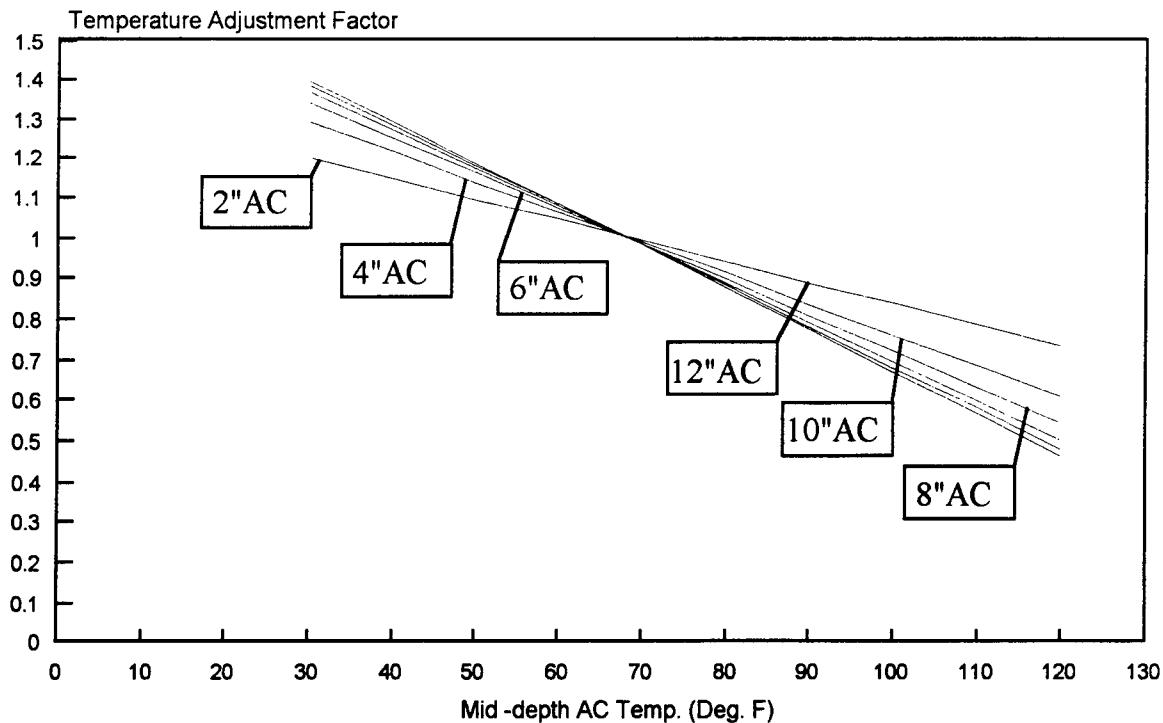
The temperature adjustment factor was calculated the same way as for the empirical TAF curve:

$$TAF = \frac{d_{0_{68}}}{d_{0_r}}$$

(See Appendix C, pages C3-C20 for all of the generated theoretical TAF data points.)

For each asphalt thickness, all the TAF data was plotted and a regression equation was computed. The regression line became the TAF curve for that asphalt thickness, just as in the empirical TAF curve procedure. (See Appendix C, page C21 for Regression Equations , pages C22-C27 for graphs of regression lines for each h_{ac} depth, and page C28 for a full page plot of Figure 6.) Below in Figure 6, a plot of all the TAF curves for the various AC thicknesses can be found.

Theoretical Temperature Adjustment Factor Curves
Figure 6



The theoretical temperature adjustment factor curves have a different curve fit than the empirical TAF curves (linear vs. exponential), but they both have the same general slope trend and the thicker the AC thickness, the steeper the slope. In the **CONCLUSIONS** section, a much more detailed comparison of the TAF curve sets can be found.

Before moving on to the comparison of methods used to find layer strengths, it should be mentioned that another method for finding theoretical deflections was examined. The method used in this investigation was based on Boussinesq's one layer system as well as on the theory of equivalent thickness presented by Odemark. Initially, Boussinesq's one layer system alone was used to find deflections. In 1885, Boussinesq developed a solution for computing stresses and deflections in a halfspace (soil) composed of homogeneous, isotropic and linearly elastic material. This solution was based on a point loading, and in

1928, Love adapted his solution for a circular load. (5) The equation below is for deflection at depth z :

$$d_z = \frac{(1+u)pa}{E} \left\{ \frac{1}{\sqrt{1+\left(\frac{z}{a}\right)^2}} + (1-2u) \left(\sqrt{1+\left(\frac{z}{a}\right)^2} - \frac{z}{a} \right) \right\}$$

where:

p = plate pressure (ksi)

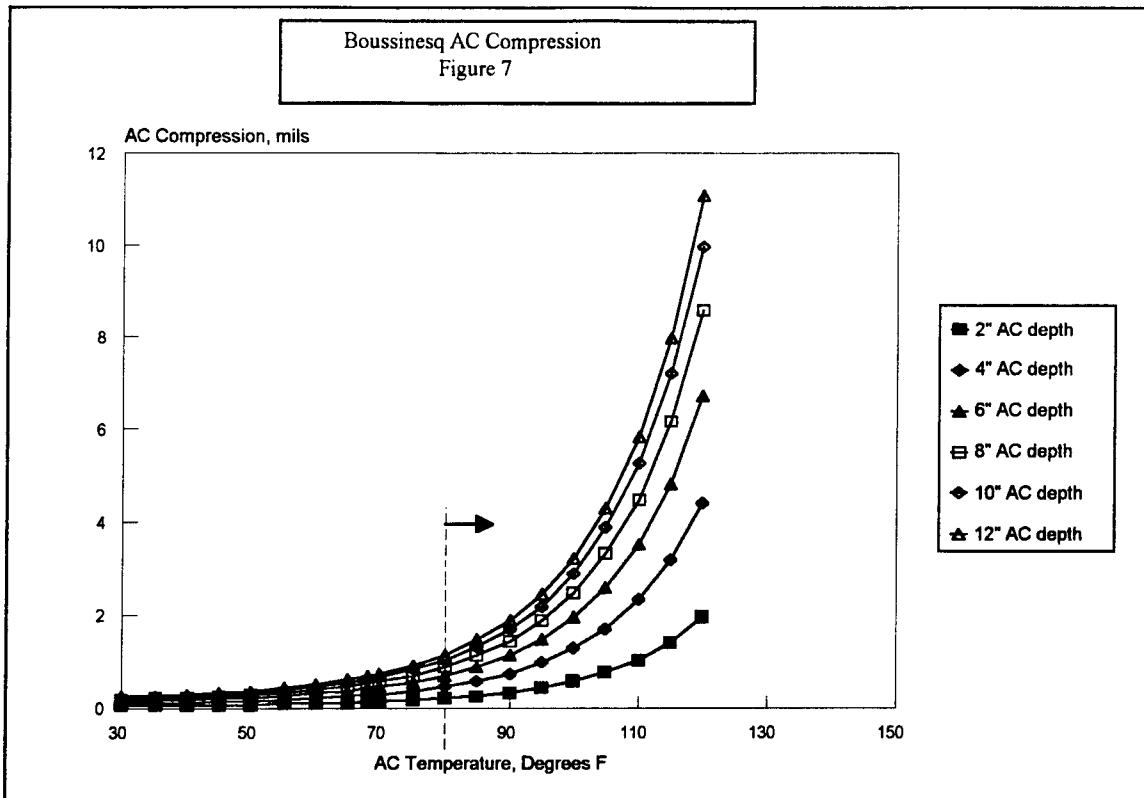
E = elastic modulus (ksi)

a = plate radius (inches)

z = depth below pavement surface (inches)

u = Poisson's ratio

The reason that this method was not used for the theoretical TAF curve procedure is that it produced inconclusive results. As can be seen from the equation above, for the same asphalt thickness, the temperature adjustment factor [TAF = d_0 at 68/ d_0 at T] is only the ratio of the moduli of elasticity, everything else cancels out. For each AC depth, then, the TAF curve was identical because the modulus of elasticity values at varying temperatures are constant for each asphalt pavement thickness. From the empirical TAF curve, it can be seen that the temperature adjustment factor curve is not the same for each AC depth, thus, the results were erroneous. What this method did show is the theoretical amount of compression the AC layer is subjected to due to different AC temperatures and layer depths. As can be seen in Figure 7 on the next page, it is an exponential curve and around the 80 degrees Fahrenheit range, the amount of compression becomes greater and greater. (See Appendix C, pages C29-C30 for the generated Boussinesq compression and TAF data in tabular form and a full page plot of Figure 7.)



Comparison of Methods Used to Find Layer Strength Moduli

As the final step in RI 91-9, methods used to find pavement layer strengths will be compared. This topic will be updated in Research Investigation 91-9B which will be completed when resilient modulus laboratory data is available. The layer strengths of concern are E_p , the strength of the whole pavement structure, E_{ac} , the strength of the asphalt, and M_r , the subgrade resilient modulus.

The MODULUS program was run three different ways with the collected FWD deflection data as inputs. The first run considered the pavement as a two layer system: an AC layer height + base layer height considered as one layer of pavement, and a subgrade. The effective pavement modulus (E_p) and the subgrade resilient modulus (M_r) found by the first MODULUS run were compared to manually backcalculated values found by equations in the *Determination of Subgrade Modulus for SN_{eff} Determination* section of the 1993

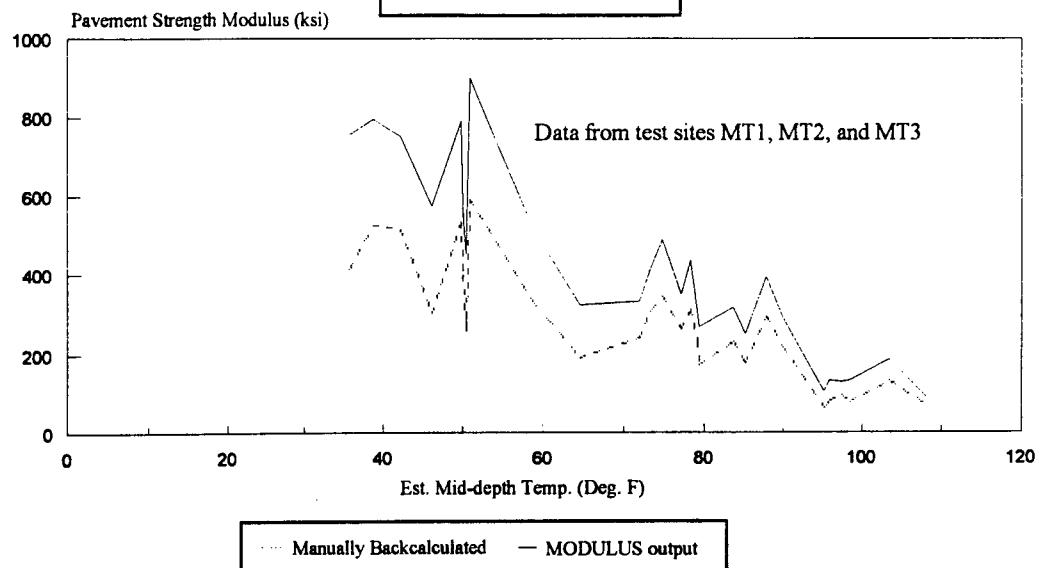
AASHTO Guide for Design of Pavement Structures. (2) This is a simple method proposed by Ultiditz for estimating the subgrade modulus from deflections measured at the surface of a layered pavement structure (by the FWD). It is based on the following two observations:

- 1) As distance away from the load increases, compression of the layers above the subgrade becomes less significant to the measured deflection at the pavement surface.
- 2) As distance away from the load increases, the approximation of a distributed load by a point load improves. (2)

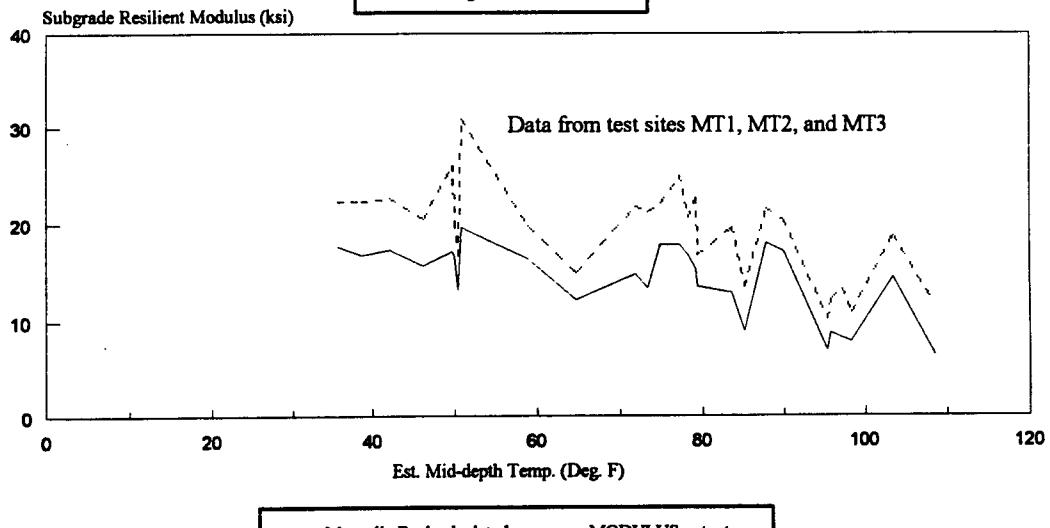
It was assumed again that the pavement structure consisted of a two layer system, therefore, the only other strength value the procedure yielded, besides M_r , was E_p . It is logical, then, to compare its outcome to the first MODULUS run. The procedure for obtaining the backcalculated E_p and M_r values as well as a sample calculation can be found in Appendix D, pages D1-D4. Plots of E_p and M_r for both methods versus estimated mid-depth temperature can be found in Figures 8 and 9 on the next page. (See Appendix D, page D5 for the data in tabular form.)

In analyzing Figures 8 and 9, it can easily be seen that the two methods are similar in the way that they find the unknown values - they both have the same trends at each mid-depth pavement temperature. The difference between the two methods is the way that they assign strengths. For the manual backcalculation method, it assigns the subgrade more of the strength and takes away from the strength E_p assumes. The opposite is true in the MODULUS program: the pavement modulus takes on the strength that is not given to the subgrade.

Ep vs. Temperature
Figure 8



Mr vs. Temperature
Figure 9

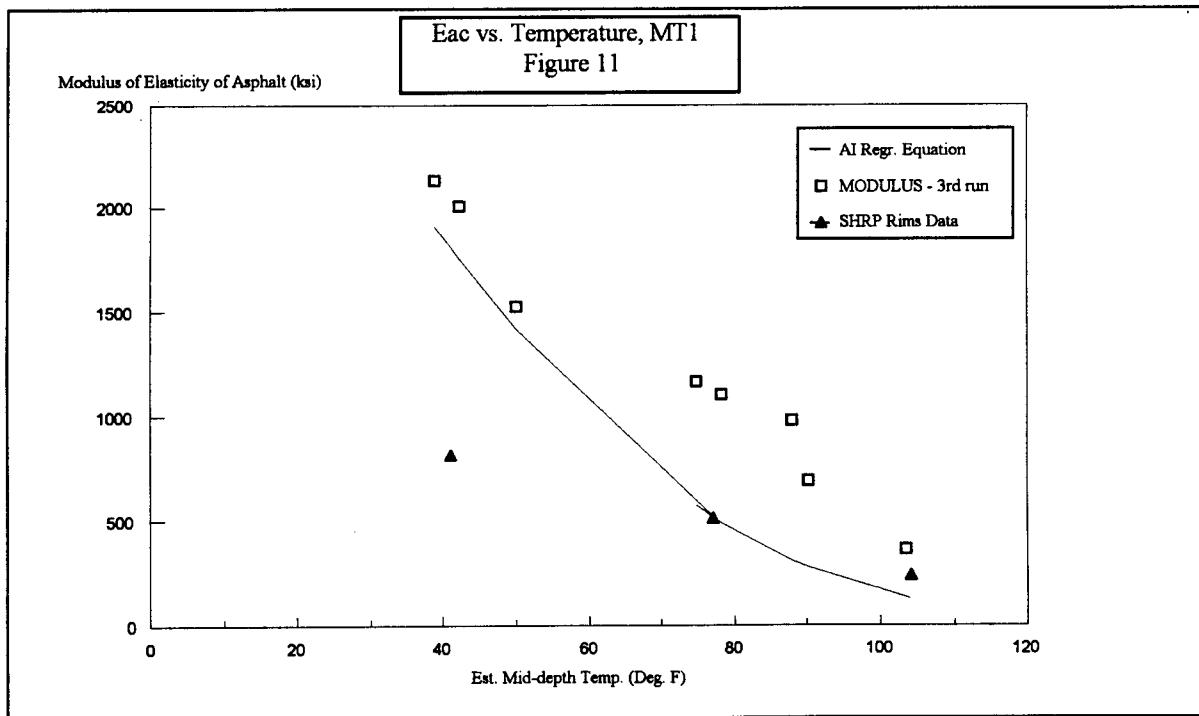
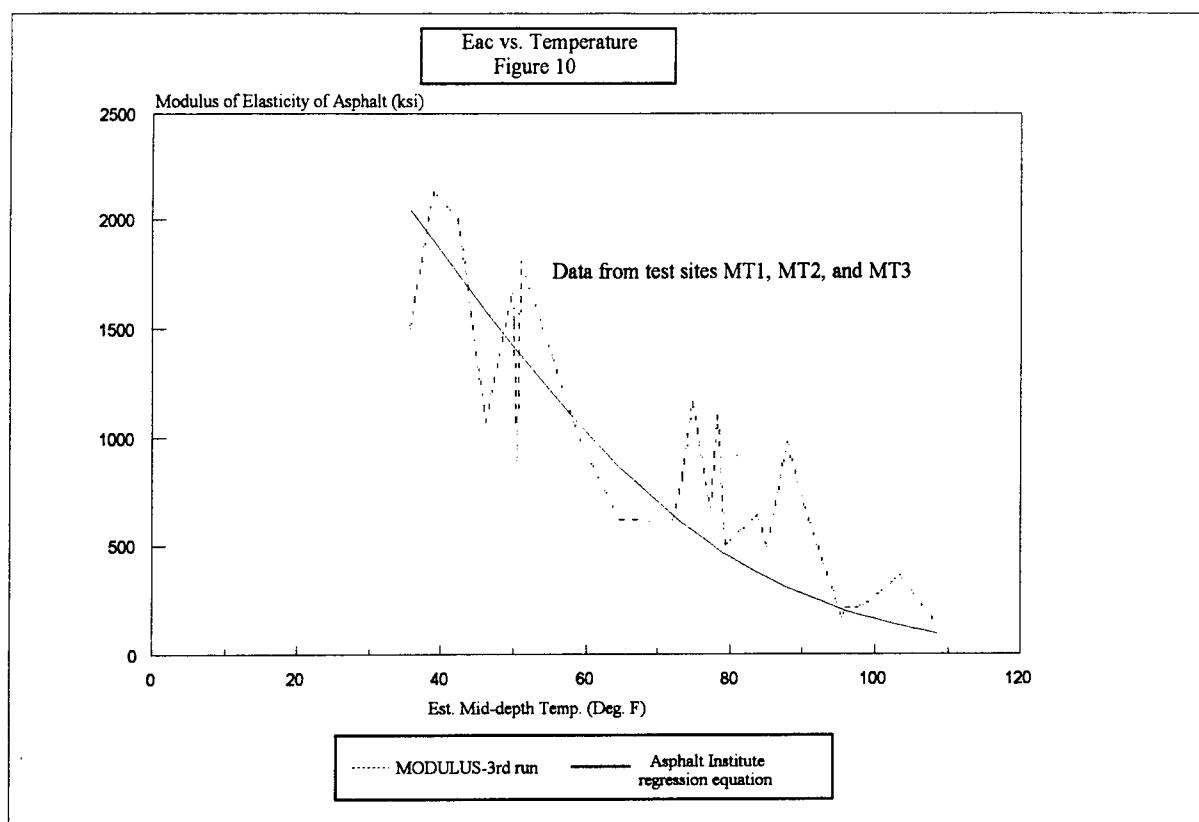


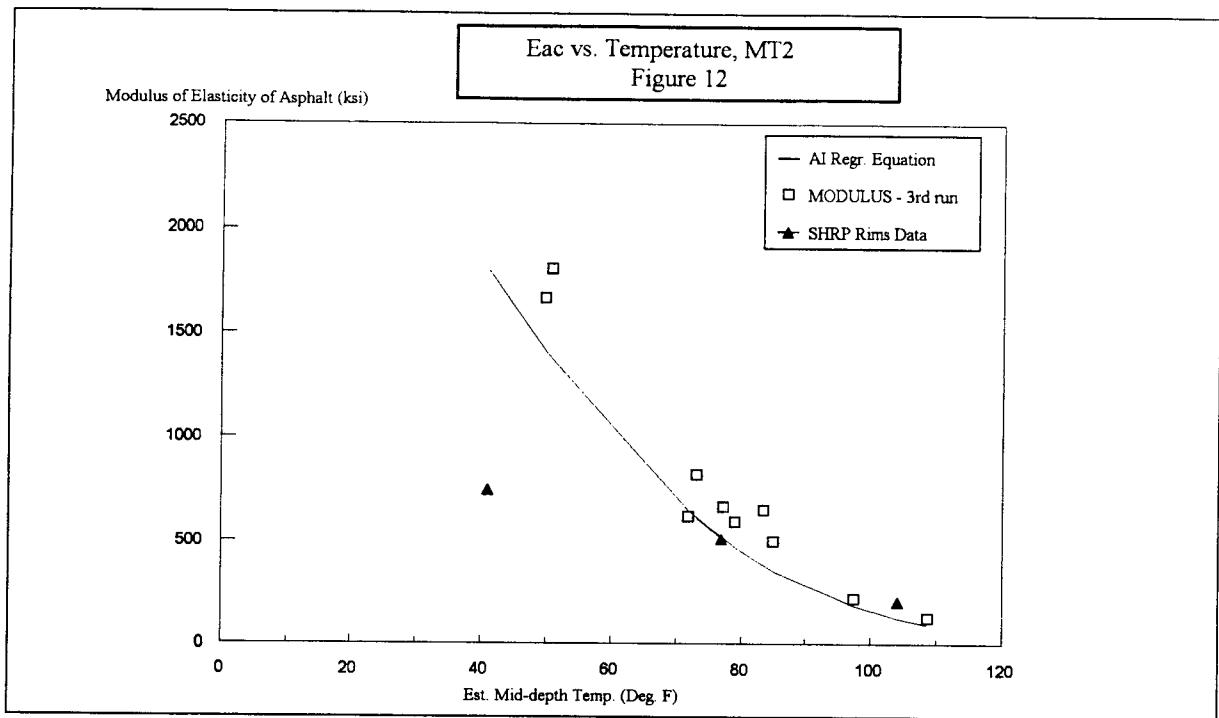
The final comparison is for E_{ac} , the asphalt concrete layer strength modulus within the pavement structure. One previously mentioned method that was used to find E_{ac} values was the Asphalt Institute Regression Equation. These values were compared to MODULUS solutions and SHRP Regional Information Management System (RIMS) laboratory data for E_{ac} .

The pavement of the test sites was analyzed as a 3 layer system (asphalt layer, base and subgrade) for the second MODULUS run and as a 4-layer system (asphalt layer, base, first 36" of subgrade and remaining subgrade) as the third MODULUS run. The third run produced an overall lower absolute error than the second run and can be justified by the fact that the first 36 inches of subgrade is the most susceptible to environmental conditions, and therefore can be figured as a separate layer. Because of the lower absolute error, the third run values were used in the subsequent comparisons. The corresponding layer strengths produced by using inputted FWD deflection data were: E_{ac} , E_b , $M_r @ 36"$, and M_r . The researcher used the resilient modulus strength of the first 36" of subgrade layer if the stiff layer was less than 10 feet below the ground surface. If the stiff layer was greater than 10 feet below the ground surface, the first 36" of subgrade resilient modulus value and the remaining subgrade resilient modulus value should be averaged.

The MODULUS third run E_{ac} values can now be compared to the Asphalt Institute regression equation values for asphalt modulus of elasticity and to the SHRP RIMS laboratory E_{ac} data. At this point, however, we have nothing to compare the E_b values with. In Figure 10 on the following page, the MODULUS E_{ac} values for each test site and the corresponding Asphalt Institute regression Modulus of Elasticity values are plotted against the estimated mid-depth temperature of the pavement. (See Appendix D, page D6 to see the all the E_{ac} data in tabular form.)

In Figures 11 and 12, asphalt Modulus of Elasticity values as determined by the Asphalt Institute regression equation, MODULUS output, and SHRP RIMS laboratory data for test sites MT1 and MT2 are plotted against the estimated mid-depth temperature.





CONCLUSIONS

The purpose of a temperature adjustment factor (TAF) is to be able to take a Falling Weight Deflectometer (FWD) deflection value measured at T degrees and multiply it by the TAF to get the equivalent deflection at 68 degrees Fahrenheit. This will provide structural data that is easily compared for various pavements.

The empirical temperature adjustment factor curves' (found in Figure 3) data points have a different curve fit than the data points of the theoretical temperature adjustment factor curves (found in Figure 6). The TAF values for the empirical data are generally higher for temperatures lower than 68 degrees and generally lower for temperatures higher than 68 degrees when compared to the theoretical temperature adjustment factor curve. This results in the empirical temperature adjustment factor curves being more conservative structurally when the temperature is above 68 degrees and less structurally conservative when the temperature is below 68 degrees.

The temperature adjustment factors which were based on theoretical results using Odemark-Boussinesq, which is a linear elastic equation, are basically the same as the present American Association of State Highway and Transportation Officials (AASHTO) Temperature Adjustment Factors on pages III-99 and III-100 of the 1993 AASHTO Guide for Design of Pavement Structures (2). Evidently, both of these results were based on linear elastic behavior of the material, which is a false assumption since the material in actuality is not linear elastic. Therefore, the temperature adjustment factors which were based on actual FWD deflection data and empirical mid-depth asphalt concrete (AC) temperatures (referred to as the empirical Temperature Adjustment Factor curves) show that the behavior is non-linear and is perceived as a more accurate illustration of the temperature adjustment factors.

The conclusions for the comparison of methods used to find layer strengths will be brief. Overall, it appears that the manually backcalculated method and 2-layer system

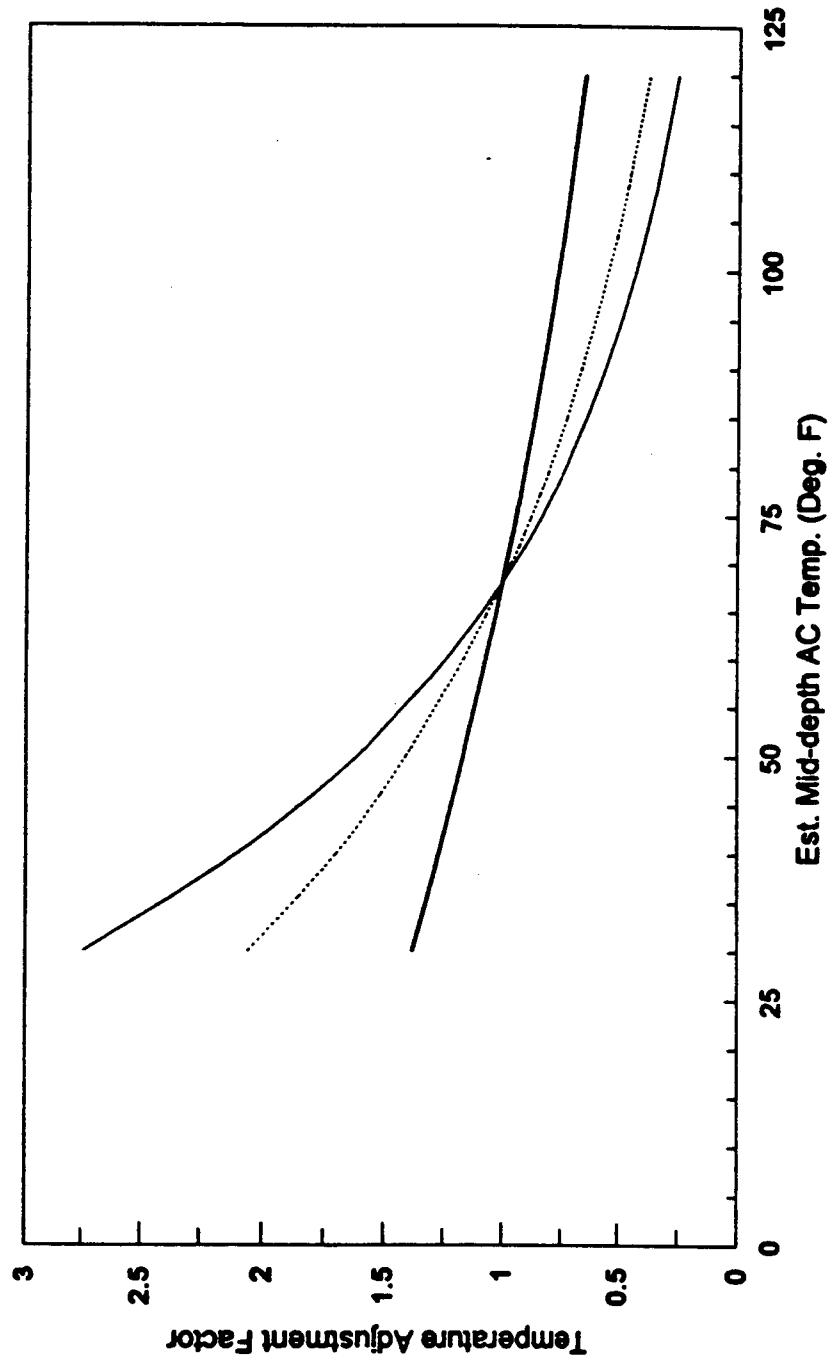
MODULUS run yield parallel values for the pavement strength moduli and the subgrade resilient modulus. (see Figures 7 and 8).

The Asphalt Institute (AI) regression equation E_{ac} (strength modulus of asphalt concrete) values provide a theoretical comparison for the 4-layer system MODULUS E_{ac} results. The MODULUS values are distributed fairly close to the AI regression line. (see Figure 10). As can be seen in Figures 11 and 12, the Modulus of Elasticity values found by the Strategic Highway Research Program (SHRP) Regional Information Management System (RIMS) laboratory tests are close to the AI and MODULUS values for warm mid-depth temperatures (over 75° Fahrenheit). But for colder mid-depth temperatures, the SHRP RIMS E_{ac} values are considerably lower. It is uncertain whether it is the SHRP laboratory data or the MODULUS software and Asphalt Institute regression results that are inconsistent, considering the large difference in asphalt layer strength modulus values. The SHRP Protocol used for the testing the total resilient modulus of asphalt concrete is PO7 which closely follows ASTM D4123 specifications.

RECOMMENDATIONS

The empirical temperature adjustment factor curves, found in Figure 13 on the following page, are the recommended Temperature Adjustment Factor curves for MoDOT's use with the FWD measured deflections. These temperature adjustment factor will be used to correct the deflections to the AASHTO standard 68 degree F. (The calculated regression equations for these lines can be found on page 27.)

Empirical Temperature Adjustment Factor Curves
Figure 13



Missouri Department of Transportation Recommended Temperature Adjustment Factor
Curve Regression Equations.

MT1 Regression FWD Deflection Equation
Correlation Coefficient = 0.895540

$$d_0 = 4.738428e^{(0.008485 * \text{TEMP})}$$

MT2 Regression FWD Deflection Equation
Correlation Coefficient = 0.962902

$$d_0 = 1.090210e^{(0.026551 * \text{TEMP})}$$

MT3 Regression FWD Deflection Equation
Correlation Coefficient = 0.950967

$$d_0 = 3.188715e^{(0.018935 * \text{TEMP})}$$

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- (1) "An Evaluation of Temperature Distribution within Asphalt Pavements and It's Relationship to Pavement Deflection", Thesis submitted in partial requirements for the degree of Master of Science in Civil Engineering at the University of Kentucky by Herbert Fletcher Southgate. Co-Directors: Dr. Bobby D. Hardin and Dr. John W. Hutchinson, Professors of Civil Engineering, Lexington, Kentucky, 1968.
- (2) *AASHTO Guide for Design of Pavement Structures, Volume 1, 1993*. American Association of State Highway and Transportation Officials, Washington, D.C. (1993).
- (3) *SHRP Procedure for Temperature Correction of Maximum Deflections*. Strategic Highway Research Program, National Research Council, Washington, D.C., 1993.
- (4) *AASHTO Guide for Design of Pavement Structures, Volume 2, 1986*. Prepared for: American Association of State Highway and Transportation Officials, Under Contract with: National Cooperative Highway Reserach Program, Transportation Research Board, National Research Council (NCHRP) Project 20-7 (Task 24 & 28)., August 1988.
- (5) *National Highway Institute FHWA Backcalculation Training Course Manual*. Athens, Ohio, October 1993

APPENDIX A

Data and Graphs Relating to the Estimation of Mid-Depth AC Temperature

Site No.	Location	Date Tested	5-day avg. temperature	Pvt. Surface temperature	5-day surf. temperature	AC. Thickness. (inches)	Actual Pavement temperature	1" depth	2" depth	4" depth	5" depth	6" depth	7" depth	8" depth	9" depth
MT1-1002	Route C, Cole County, MO Asphalt Pavement	2-27-91 5-01-91 6-06-91 7-05-91 7-29-91 9-05-91 10-03-91 11-12-91 2-11-92 3-19-92 5-18-92	40 52 77 81 72 74 68 66 26 31 43 65	40 52 77 86 106 85 85 68 52 41 49 77	163 187 157 159 85 136 78 52 72 82 77	7 7 7 7 7 7 7 7 7 7 7	64 68 92 103 82 82 68 51 41 39 40 74	64 68 92 105 82 82 70 51 47 39 54 72	63 87 98 105 88 88 77 70 47 39 52 72	49 87 98 105 88 88 77 70 47 39 52 72	52 52 52 52 52 52 52 52 52 52 52 52	52 52 52 52 52 52 52 52 52 52 52 52	49 87 98 105 88 88 77 70 47 39 52 72	49 87 98 105 88 88 77 70 47 39 52 72	49 87 98 105 88 88 77 70 47 39 52 72

Site No.	Location	Date Tested	5-day avg. temperature	Pvt. Surface temperature	5-day surf. temperature	AC. Thickness. (inches)	Actual Pavement Temperature	1" depth	2" depth	4" depth	5" depth	6" depth	7" depth	8" depth	9" depth	
MT2-1008	Route 171, Jasper County, MO Asphalt Pavement	2-26-91 3-26-91 4-24-91 5-30-91 6-25-91 7-22-91 9-04-91 10-01-91 10-29-91 1-27-92 3-24-92 5-20-94	43 64 50 78 79 84 77 75 63 38 58 42 70	43 64 50 78 79 84 77 75 63 38 58 42 70	88 140 137 164 189 212 152 148 117 94 92 91	10 10 10 10 10 10 10 10 10 10 10 10	45 73 93 85 113 128 75 83 63 56 56 50	42 69 76 83 100 118 80 80 70 63 46 48								

Site No.	Location	Date Tested	5-day avg. temperature	Pvt. Surface temperature	5-day surf. temperature	AC. Thickness. (inches)	Actual Pavement Temperature	1" depth	2" depth	4" depth	5" depth	6" depth	7" depth	8" depth	9" depth
MT3-606	Route 169, Gentry County, MO Asphalt Pavement	3-05-91 4-03-91 5-01-91 6-04-91 7-02-91 7-30-91 9-10-91 10-09-91 11-05-91 1-30-92 3-31-92 5-27-92	41 44 58 76 82 71 76 54 20 54 54 53	41 44 58 76 82 71 75 58 48 54 54 53	9 9 9 181 187 182 151 112 68 88 96 123	61 78 63 110 9 113 76 53 9 55 9 75	48 77 55 99 96 99 78 42 53 65								

Site No.	Location	Date Tested	5-day avg temperature	Pvt. Surface temperature	5-day+surf. temperature	AC Thickness (inches)	Actual Pavement Temperature						Gr. depth	
							1" depth	2" depth	4" depth	5" depth	6" depth	7" depth	8" depth	
M14-5000	I-35, Daviess County, MO Concrete Pavement	3-05-91	36	-	-	9.2	60	60	84	54	78	-	-	45
		4-02-91	43	-	-	9.2	9.2	9.2	85	92	80	92	-	69
		4-30-91	57	-	-	9.2	9.2	9.2	89	92	92	96	-	84
		6-04-91	76	85	161	9.2	9.2	9.2	69	102	102	102	-	88
		7-03-91	80	91	171	9.2	9.2	9.2	69	62	62	62	-	83
		7-30-91	71	69	140	9.2	9.2	9.2	69	35	35	35	-	90
		9-09-91	72	101	173	9.2	9.2	9.2	62	59	59	59	-	63
		10-08-91	51	63	114	9.2	9.2	9.2	56	103	103	103	-	40
		11-06-91	23	33	56	9.2	9.2	9.2	59	60	60	60	-	42
		1-29-92	33	60	93	9.2	9.2	9.2	56	56	56	56	-	49
M15-9005	Route 50, Cole County, MO Concrete Pavement	3-31-92	43	60	103	9.2	9.2	9.2	56	56	56	56	-	60
		5-28-92	53	66	119	9.2	9.2	9.2	64	64	64	64	-	60
		6-06-91	42	-	-	9	9	9	65	77	77	77	-	58
		3-28-91	62	-	-	9	9	9	73	65	65	65	-	61
		6-03-91	79	92	171	9	9	9	89	89	89	89	-	85
		7-05-91	81	97	118	9	9	9	95	95	95	95	-	87
		7-29-91	72	81	153	9	9	9	81	81	81	81	-	84
		9-05-91	74	96	170	9	9	9	96	96	96	96	-	82
		10-03-91	66	110	176	9	9	9	86	86	86	86	-	73
		11-12-91	26	42	68	9	9	9	42	42	42	42	-	44
M16-9007	Route 71, Jasper County, MO Concrete Pavement	2-11-92	31	40	71	9	9	9	40	38	38	38	-	39
		4-01-92	45	51	96	9	9	9	54	54	54	54	-	52
		5-18-92	64	89	153	9	9	9	91	91	91	91	-	76
		2-25-91	43	-	-	9.3	9.3	9.3	92	92	92	92	-	49
		3-25-91	59	-	-	9.3	9.3	9.3	86	86	86	86	-	73
		4-29-91	63	-	-	9.3	9.3	9.3	70	70	70	70	-	73
		5-30-91	77	-	-	9.3	9.3	9.3	75	97	97	97	-	86
		6-25-91	79	97	176	9.3	9.3	9.3	88	88	88	88	-	86
		7-23-91	86	87	173	9.3	9.3	9.3	92	92	92	92	-	90
		9-04-91	78	93	111	9.3	9.3	9.3	84	84	84	84	-	83
M17-9007	Route 71, Jasper County, MO Concrete Pavement	10-30-91	62	40	102	9.3	9.3	9.3	45	48	48	48	-	52
		1-28-92	37	43	80	9.3	9.3	9.3	48	47	47	47	-	41
		5-20-92	62	77	139	9.3	9.3	9.3	76	76	76	76	-	73

Site No.	Location	Date Tested	5-day avg. temperature	Pvt. Surface temperature	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavmt Temperature
MT7-8013	I-35, Harrison County, MO Asphalt Overlay on Concrete Pavement	3-05-91 4-03-91 5-02-91 6-04-91 7-02-91 7-30-91 9-10-91 10-08-91 11-05-91 1-30-92 3-30-92 5-27-92	36 45 58 76 82 70 73 50 23 30 42 75 61	- - - - - - - 77 36 33 117 120	4 4 162 162 163 155 127 59 63 4 4	4 4 4 4 4 4 4 4 4 4 4	51 56 57 90 88 93 85 78 32 33 70 60

Site No.	Location	Date Tested	5-day avg. temperature	Pvt. Surface temperature	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavmt Temp.
MT8-5473	I-70, Cooper County, MO Asphalt Overlay on Concrete Pavement	3-04-91 4-01-91 5-09-91 6-30-91 7-01-91 7-29-91 9-06-91 10-18-91 11-13-91 1-31-92 3-20-92 5-22-92	41 49 60 79 82 72 74 56 28 36 42 69	- - - - - 102 96 101 81 61 53 57 43 77	181 178 173 155 117 81 93 85 146	3 3 3 3 3 3 3 3 3 3	26 68 72 102 96 100 75 66 46 53 45 78

Site No.	Location	Date Tested	5-day avg. temperature	Pvt. Surface temperature	5-day+surf. temperature	AC. Thickns. (inches)	Actual Pavmt Temp.
MT9-8014	Route 60, Greene County, MO Asphalt Overlay on Concrete Pavement	2-26-91 3-19-91 4-29-91 5-31-91 6-26-91 7-22-91 9-03-91 9-30-91 11-14-91 1-27-92 3-23-92 5-19-92	38 41 61 75 77 77 75 59 42 34 47 67	- - - - - - 95 95 56 54 64 77	3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5 3.5	57 71 84 81 109 113 95 82 56 50 62 75	

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Concrete pvt. surf. temp.	5-day+surf. temperature	AC. Thickness (inches)	Actual Pavement Temperature at depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth
SPS-6 Bethany, MO	1-Control Section	6/02/92	57.3	87	144.3	8	87	78	79	81	81	79	79
		6/02/92	57.3	87	144.3	8	87	88	88	88	81	81	79
		6/02/92	57.3	88	145.3	8	88	88	88	88	81	81	79
		6/02/92	57.3	88	145.3	8	88	88	88	88	81	81	79
		6/02/92	57.3	90	147.3	8	90	90	90	90	83	83	81
		6/02/92	57.3	93	150.3	8	93	93	93	93	84	84	80
		6/02/92	57.3	90	147.3	8	90	90	90	90	84	84	80
		10/19/92	47.3	53	100.3	8	53	53	53	53	54	54	54
		10/19/92	47.3	52	99.3	8	52	52	52	52	53	53	54
		10/19/92	47.3	53	100.3	8	53	53	53	53	54	54	54
		10/19/92	47.3	51	98.3	8	51	51	51	52	52	53	53
		10/19/92	47.3	52	99.3	8	52	52	52	52	51	51	52
		10/19/92	47.3	49	96.3	8	49	49	49	49	50	50	51
		10/19/92	47.3	50	97.3	8	50	50	50	50	51	51	52
		10/28/92	60.3	55	115.3	8	55	55	55	55	56	56	57
		10/28/92	60.3	57	117.3	8	57	57	57	57	56	56	58
		10/28/92	60.3	57	117.3	8	57	57	57	57	56	56	58
		10/28/92	60.3	61	121.3	8	61	61	61	61	58	58	59
		10/28/92	60.3	63	123.3	8	63	63	63	63	60	60	59
		10/28/92	60.3	61	121.3	8	61	61	61	61	60	60	61
		10/28/92	60.3	63	123.3	8	63	63	63	63	63	63	67
		11/17/92	37.9	54	91.9	8	54	54	54	54	52	52	49
		11/17/92	37.9	54	91.9	8	54	54	54	54	51	51	51
		11/17/92	37.9	51	88.9	8	51	51	51	51	50	50	50
		11/17/92	37.9	47	84.9	8	47	47	47	47	47	47	49
		11/17/92	37.9	46	83.9	8	46	46	46	46	43	43	43
		11/17/92	37.9	39	76.9	8	39	39	39	39	73	73	67
		4/22/93	50.1	77	127.1	8	77	77	77	77	75	75	69
		4/22/93	50.1	79	129.1	8	79	79	79	79	78	78	72
		4/22/93	50.1	82	132.1	8	82	82	82	82	80	80	74
		4/22/93	50.1	84	134.1	8	84	84	84	84	78	78	73
		4/22/93	50.1	80	130.1	8	80	80	80	80	75	75	71
		4/22/93	50.1	79	129.1	8	79	79	79	79	73	73	71
		4/22/93	50.1	77	127.1	8	77	77	77	77	73	73	71
		4/22/93	50.1	73	123.1	8	73	73	73	73	73	73	73
		5/10/93	65.6	74	139.6	8	74	74	74	74	73	73	73
		5/10/93	65.6	72	137.6	8	72	72	72	72	73	73	73

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Concrete pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature	1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth
SPS-6 Bethany, MO	2 - Max. Restoration	3/12/92	46.8	40	86.8	8	40	33	30	34	34	34	35	42	46
		3/12/92	46.8	33	79.8	8	33	30	32	32	32	32	34	35	37
		3/12/92	46.8	30	76.8	8	30	28	28	28	28	28	28	30	37
		3/12/92	46.8	32	78.8	8	32	30	30	30	30	30	30	30	37
		4/15/92	49.1	68	117.1	8	68	68	68	68	68	68	65	65	64
		4/15/92	49.1	71	120.1	8	71	71	71	71	71	71	67	67	67
		4/15/92	49.1	75	124.1	8	75	75	75	75	75	75	71	71	67
		4/15/92	49.1	77	126.1	8	77	77	77	77	77	77	72	72	68
		4/15/92	49.1	81	130.1	8	81	81	81	81	81	81	76	76	71
		4/15/92	49.1	87	136.1	8	87	87	87	87	87	87	80	80	75
		4/15/92	49.1	89	138.1	8	89	89	89	89	89	89	83	83	77
		4/15/92	49.1	91	140.1	8	91	91	91	91	91	91	84	84	78
		4/15/92	49.1	94	143.1	8	94	94	94	94	94	94	86	86	79
		4/15/92	49.1	95	144.1	8	95	95	95	95	95	95	87	87	80
		4/22/92	50.1	33	83.1	8	33	33	33	33	33	33	33	33	38
		4/22/92	50.1	39	89.1	8	39	39	39	39	39	39	36	36	36
		4/22/92	50.1	46	96.1	8	46	46	46	46	46	46	40	40	37
		4/22/92	50.1	50	100.1	8	50	50	50	50	50	50	46	46	39
		4/22/92	50.1	53	103.1	8	53	53	53	53	53	53	48	48	46
		10/27/92	63	46	109	8	46	46	46	46	46	46	48	48	53
		10/27/92	63	49	112	8	49	49	49	49	49	49	49	49	53
		10/27/92	63	50	113	8	50	50	50	50	50	50	52	52	53
		10/27/92	63	55	118	8	55	55	55	55	55	55	55	55	54
		10/27/92	63	62	125	8	62	62	62	62	62	62	57	57	56
		10/27/92	63	68	131	8	68	68	68	68	68	68	64	64	62
		10/27/92	63	66	129	8	66	66	66	66	66	66	62	62	56
		10/27/92	63	58	121	8	58	58	58	58	58	58	58	58	68
		10/27/92	63	65	128	8	65	65	65	65	65	65	57	57	65
		10/27/92	63	73	136	8	73	73	73	73	73	73	68	68	61
		10/27/92	63	72	135	8	72	72	72	72	72	72	66	66	61
		10/27/92	63	70	133	8	70	70	70	70	70	70	67	67	62
		10/27/92	63	67	130	8	67	67	67	67	67	67	67	67	64
		5/11/93	65.8	64	129.8	8	64	64	64	64	64	64	63	63	64
		5/11/93	65.8	65	130.8	8	65	65	65	65	65	65	65	65	64
		5/11/93	65.8	66	131.8	8	66	66	66	66	66	66	66	66	65
		5/11/93	65.8	67	132.8	8	67	67	67	67	67	67	67	67	65
		5/11/93	65.8	66	131.8	8	66	66	66	66	66	66	66	66	65
		5/11/93	65.8	69	134.8	8	69	69	69	69	69	69	68	68	66
		5/11/93	65.8	71	136.8	8	71	71	71	71	71	71	71	71	65
		5/11/93	65.8	73	138.8	8	73	73	73	73	73	73	72	72	65
		5/11/93	65.8	74	139.8	8	74	74	74	74	74	74	73	73	69
		5/11/93	65.8	71	136.8	8	71	71	71	71	71	71	70	70	68
		5/11/93	65.8	71	136.8	8	71	71	71	71	71	71	70	70	68
		5/11/93	65.8	69	134.8	8	69	69	69	69	69	69	69	69	68
		5/11/93	65.8	70	135.8	8	70	70	70	70	70	70	70	70	69
		5/11/93	65.8	75	140.8	8	75	75	75	75	75	75	71	71	69

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature 1" depth	2" depth	3" depth	4" depth
SPS-6 Bethany, MO	3 - min. restoration	3/3/92	46.1	70	116.1	4	70			
		3/3/92	46.1	72	118.1	4	72			
		3/3/92	46.1	71	117.1	4	71			
		3/3/92	46.1	73	119.1	4	73			
		3/3/92	46.1	73	119.1	4	73			
		3/3/92	46.1	73	119.1	4	73			
		3/4/92	51.4	59	110.4	4	59			
		3/4/92	51.4	60	111.4	4	60			
		3/4/92	51.4	65	116.4	4	65			
		3/4/92	51.4	66	117.4	4	66			
		3/4/92	51.4	70	121.4	4	70			
		10/21/92	41.1	87	128.1	4	87			
		10/21/92	41.1	88	129.1	4	88			
		10/21/92	41.1	87	128.1	4	87			
		10/21/92	41.1	84	125.1	4	84			
		10/21/92	41.1	83	124.1	4	83			
		10/21/92	41.1	82	123.1	4	82			
		10/22/92	44.2	59	103.2	4	59			
		10/22/92	44.2	61	105.2	4	61			
		10/22/92	44.2	63	107.2	4	63			
		10/22/92	44.2	64	108.2	4	64			
		10/22/92	44.2	69	113.2	4	69			
		10/22/92	44.2	64	108.2	4	64			
		10/22/92	44.2	75	119.2	4	75			
		5/11/93	65.8	78	143.8	4	78			
		5/11/93	65.8	80	145.8	4	80			
		5/11/93	65.8	79	144.8	4	79			
		5/11/93	65.8	74	139.8	4	74			
		5/12/93	65.2	66	131.2	4	66			
		5/12/93	65.2	70	135.2	4	70			
		5/12/93	65.2	70	135.2	4	70			
		5/12/93	65.2	79	144.2	4	79			
		5/12/93	65.2	78	143.2	4	78			

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature			
							1" depth	2" depth	3" depth	4" depth
SPS-6 Bethany, MO	4 - min. restoration (saw & seal overlay)	3/02/92	39.6	99	138.6	4	99	86	84	58
		3/02/92	39.6	86	125.6	4	4	84	84	60
		3/02/92	39.6	84	123.6	4	4	60	60	64
		3/03/92	46.1	58	104.1	4	4	64	64	66
		3/03/92	46.1	60	106.1	4	4	66	66	66
		3/03/92	46.1	64	110.1	4	4	67	67	70
		3/03/92	46.1	66	112.1	4	4	71	71	71
		3/03/92	46.1	66	112.1	4	4	72	72	72
		3/03/92	46.1	67	113.1	4	4	71	71	71
		3/03/92	46.1	70	116.1	4	4	70	70	70
		3/03/92	46.1	71	117.1	4	4	71	71	71
		3/03/92	46.1	72	118.1	4	4	72	72	72
		3/03/92	46.1	71	117.1	4	4	71	71	71
		3/03/92	46.1	71	92.1	4	4	51	51	51
		10/21/92	41.1	51	95.1	4	4	54	54	53
		10/21/92	41.1	54	95.1	4	4	57	57	56
		10/21/92	41.1	57	98.1	4	4	60	60	60
		10/21/92	41.1	60	101.1	4	4	65	65	62
		10/21/92	41.1	65	106.1	4	4	70	70	68
		10/21/92	41.1	70	111.1	4	4	74	74	73
		10/21/92	41.1	74	115.1	4	4	75	75	74
		10/21/92	41.1	75	116.1	4	4	78	78	78
		10/21/92	41.1	78	119.1	4	4	83	83	82
		10/21/92	41.1	83	124.1	4	4	85	85	84
		10/21/92	41.1	85	126.1	4	4	85	85	85
		10/21/92	41.1	85	126.1	4	4	56	56	57
		4/20/93	45.9	56	101.9	4	4	57	57	58
		4/20/93	45.9	57	102.9	4	4	59	59	58
		4/20/93	45.9	59	104.9	4	4	60	60	58
		4/20/93	45.9	60	105.9	4	4	60	60	59
		4/20/93	45.9	62	147.8	4	4	82	82	81
		5/11/93	65.8	84	149.8	4	4	84	84	83

Location	Test Section No.	Date Tested	5-day avg. temperature	Concrete pvt. surf. temp.	5-day*surf. temperature	AC Thickness (inches)	Actual Pavement Temperature	1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth
SPS-6 Bethany, MO	5 - Max Restoration	3/10/92	57.5	45	102.5	8	45	42	42	41	41	44	41	41	43
		3/11/92	57.5	42	99.5	8	42	42	43	43	42	42	42	42	42
		3/10/92	57.5	43	100.5	8	43	43	43	42	42	42	42	42	42
		3/10/92	57.5	45	102.5	8	45	45	45	46	46	46	46	46	43
		3/10/92	57.5	46	103.5	8	46	46	46	46	46	46	46	46	45
		3/10/92	57.5	48	105.5	8	48	48	48	47	47	47	47	47	47
		3/10/92	57.5	47	104.5	8	47	47	47	47	47	47	47	47	46
		3/10/92	57.5	48	105.5	8	48	48	48	47	47	47	47	47	47
		3/10/92	57.5	48	105.5	8	48	48	48	47	47	47	47	47	48
		3/11/92	53.6	29	92.6	8	29	29	29	31	31	31	31	31	35
		3/11/92	53.6	30	83.6	8	30	30	30	32	32	32	32	32	35
		3/11/92	53.6	30	83.6	8	30	30	30	31	31	31	31	31	35
		3/11/92	53.6	36	89.6	8	36	36	36	36	36	36	36	36	36
		3/11/92	53.6	41	94.6	8	41	41	41	41	41	41	41	41	37
		3/11/92	53.6	43	96.6	8	43	43	43	42	42	42	42	42	39
		3/11/92	53.6	44	97.6	8	44	44	44	42	42	42	42	42	39
		3/11/92	53.6	42	95.6	8	42	42	42	41	41	40	40	40	40
		3/11/92	53.6	41	94.6	8	41	41	41	40	40	40	40	40	40
		10/20/92	43.5	54	97.5	8	54	54	54	54	54	54	54	54	53
		10/20/92	43.5	56	99.5	8	56	56	56	55	55	55	55	55	54
		10/20/92	43.5	58	101.5	8	58	58	58	57	57	57	57	57	56
		10/20/92	43.5	61	104.5	8	61	61	61	60	60	60	60	60	55
		10/20/92	43.5	64	107.5	8	64	64	64	63	63	63	63	63	54
		10/20/92	43.5	63	106.5	8	63	63	63	62	62	62	62	62	55
		10/20/92	43.5	64	107.5	8	64	64	64	63	63	63	63	63	55
		10/20/92	43.5	64	107.5	8	64	64	64	64	64	64	64	64	55
		10/20/92	43.5	64	107.5	8	64	64	64	64	64	64	64	64	55
		10/20/92	43.5	75	118.5	8	75	75	75	74	74	74	74	74	66
		10/20/92	43.5	77	120.5	8	77	77	77	76	76	76	76	76	66
		10/20/92	43.5	80	123.5	8	80	80	80	79	79	79	79	79	66
		10/20/92	43.5	81	124.5	8	81	81	81	80	80	80	80	80	66
		10/20/92	43.5	84	127.5	8	84	84	84	83	83	83	83	83	66
		10/20/92	43.5	81	124.5	8	81	81	81	80	80	80	80	80	66
		10/20/92	43.5	80	123.5	8	80	80	80	79	79	79	79	79	66
		10/20/92	43.5	77	117.5	8	77	77	77	76	76	76	76	76	66
		10/20/92	43.5	73	116.5	8	73	73	73	72	72	72	72	72	66
		10/20/92	43.5	68	111.5	8	68	68	68	67	67	67	67	67	64
		10/20/92	43.5	68	121.5	8	73	73	73	72	72	72	72	72	64
		10/20/92	43.5	66	109.5	8	66	66	66	65	65	65	65	65	64
		10/26/92	62.1	72	134.1	8	72	72	72	71	71	71	71	71	67
		10/26/92	62.1	73	135.1	8	73	73	73	72	72	72	72	72	67
		10/26/92	62.1	71	133.1	8	71	71	71	70	70	70	70	70	67
		10/26/92	62.1	67	129.1	8	67	67	67	66	66	66	66	66	69
		4/21/93	48.5	73	121.5	8	73	73	73	72	72	72	72	72	69
		4/21/93	48.5	72	120.5	8	72	72	72	71	71	71	71	71	69
		4/21/93	48.5	70	118.5	8	70	70	70	69	69	69	69	69	68
		4/21/93	50.1	50	100.1	8	50	50	50	49	49	49	49	49	59
		4/22/93	50.1	54	104.1	8	54	54	54	53	53	53	53	53	61
		4/22/93	50.1	57	107.1	8	57	57	57	56	56	56	56	56	62
		4/22/93	50.1	61	114.1	8	61	61	61	60	60	60	60	60	63
		4/22/93	50.1	69	119.1	8	69	69	69	68	68	68	68	68	68
		4/22/93	50.1	72	122.1	8	72	72	72	71	71	71	71	71	69
		4/22/93	50.1	75	125.1	8	75	75	75	74	74	74	74	74	67
		4/22/93	50.1	78	128.1	8	78	78	78	77	77	77	77	77	69
		4/22/93	50.1	80	130.1	8	80	80	80	79	79	79	79	79	70
		4/22/93	50.1	81	131.1	8	81	81	81	80	80	80	80	80	71
		4/22/93	56.4	66	122.4	8	66	66	66	65	65	65	65	65	62
		4/22/93	56.4	67	123.4	8	67	67	67	66	66	66	66	66	63
		4/22/93	56.4	70	126.4	8	70	70	70	69	69	69	69	69	64
		4/22/93	56.4	72	128.4	8	72	72	72	71	71	71	71	71	64
		4/22/93	56.4	67	123.4	8	67	67	67	66	66	66	66	66	64
		4/28/93	56.4	65	121.4	8	65	65	65	64	64	64	64	64	64

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature			
							1" depth	2" depth	3" depth	4" depth
SPS-6 Bethany, MO	6 - Max. Restoration	3/04/92	51.4	68	119.4	4	68			
		3/04/92	51.4	51.4	102.8	4	70			
		3/04/92	51.4	51.4	102.8	4	68			
		3/04/92	51.4	51.4	102.8	4	64			
		3/04/92	51.4	51.4	102.8	4	65			
		3/04/92	51.4	51.4	102.8	4	65			
		3/04/92	51.4	51.4	102.8	4	65			
		3/09/92	57.8	46	103.8	4	46			
		3/09/92	57.8	47	104.8	4	47			
		3/09/92	57.8	45	102.8	4	45			
		3/09/92	57.8	42	99.8	4	42			
		3/09/92	57.8	42	99.8	4	42			
		12/01/92	27.5	37	64.5	4	37			
		12/01/92	27.5	37	64.5	4	37			
		12/01/92	27.5	37	64.5	4	37			
		12/01/92	27.5	37	64.5	4	37			
		12/01/92	27.5	37	64.5	4	37			
		12/01/92	27.5	38	65.5	4	38			
		12/01/92	27.5	39	66.5	4	39			
		12/01/92	27.5	40	67.5	4	40			
		12/01/92	27.5	41	68.5	4	41			
		12/02/92	27.5	36	63.5	4	36			
		12/02/92	27.5	36	63.5	4	36			
		5/12/93	65.2	79	144.2	4	79			
		5/12/93	65.2	82	147.2	4	82			
		5/12/93	65.2	86	151.2	4	86			
		5/12/93	65.2	95	160.2	4	95			
		5/12/93	65.2	99	164.2	4	99			
		5/12/93	65.2	104	169.2	4	104			
		5/12/93	65.2	102	167.2	4	102			
		5/12/93	65.2	102	167.2	4	102			

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature 1" depth	2" depth	3" depth	4" depth
SPS-6 Bethany, MO	7 - Crack/Break & Seat	4/24/92	45.8	50	95.8	4	50	55	59	59
		4/24/92	45.8	52	97.8	4	52	56	59	59
		4/24/92	45.8	53	98.8	4	53	56	59	59
		4/27/92	41.3	79	120.3	4	79	74	65	65
		4/27/92	41.3	80	121.3	4	80	75	62	62
		4/27/92	41.3	82	123.3	4	82	77	64	64
		4/27/92	41.3	84	125.3	4	84	78	66	66
		4/27/92	41.3	84	125.3	4	84	79	65	65
		4/28/92	43.6	52	95.6	4	52	52	55	55
		4/28/92	43.6	52	95.6	4	52	52	55	55
		4/28/92	43.6	54	97.6	4	54	54	55	55
		4/28/92	43.6	55	98.6	4	55	55	56	56
		4/28/92	43.6	59	102.6	4	59	56	56	56
		4/28/92	43.6	64	107.6	4	64	56	50	50
		4/28/92	43.6	40	86.5	4	40	40	40	40
		11/03/92	46.5	41	87.5	4	41	40	40	40
		11/03/92	46.5	40	86.5	4	40	40	41	41
		11/03/92	46.5	40	86.5	4	40	40	41	41
		11/03/92	46.5	40	86.5	4	41	41	41	41
		11/03/92	46.5	41	87.5	4	41	48	48	48
		4/06/93	36.2	48	84.2	4	48	48	48	48
		4/06/93	36.2	50	86.2	4	50	49	49	49
		4/06/93	36.2	51	87.2	4	51	50	49	49
		4/06/93	36.2	58	94.2	4	58	55	52	52
		4/06/93	36.2	57	93.2	4	57	57	55	55
		4/06/93	36.2	68	104.2	4	68	64	60	60
		4/08/92	39.4	50	89.4	4	50	51	50	50
		4/08/92	39.4	53	92.4	4	53	54	54	54
		4/08/92	39.4	53	92.4	4	53	53	53	53
		9/20/92	51.6	92	143.6	4	92	90	86	86
		9/20/92	51.6	95	146.6	4	95	93	88	88
		9/20/92	51.6	97	148.6	4	97	96	93	93
		9/20/92	51.6	92	143.6	4	92	94	91	91
		9/20/92	51.6	91	142.6	4	91	90	89	89
		9/20/92	51.6	88	139.6	4	88	89	89	89

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pH surf. temp.	5-day/surf temperature	AC Thickness (inches)	Actual Pavement Temperature	1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth	9" depth	10" depth	11" depth	12" depth
SPS-6 Bethany, MO	B Crack/Break & Seal	4/26/92	43.6	68	111.6	8	68	68	80	80	82	83	83	83	83	83	84	64	64
		4/26/92	43.6	80	123.6	8	76	76	76	76	76	76	76	76	76	76	73	71	71
		4/26/92	43.6	76	119.6	8	80	80	80	80	80	80	80	80	80	80	74	74	74
		4/26/92	43.6	80	123.6	8	82	82	82	82	82	82	82	82	82	82	76	76	76
		4/26/92	43.6	82	125.6	8	82	82	82	82	82	82	82	82	82	82	76	76	76
		4/28/92	43.6	83	126.6	8	83	83	83	83	83	83	83	83	83	83	76	76	76
		4/28/92	43.6	85	128.6	8	85	85	85	85	85	85	85	85	85	85	79	79	79
		4/28/92	43.6	78	121.6	8	82	82	82	82	82	82	82	82	82	82	78	75	75
		4/28/92	43.6	82	125.6	8	82	82	82	82	82	82	82	82	82	82	82	82	82
		4/21/93	48.5	50	98.5	8	50	50	50	50	50	50	50	50	50	50	50	50	50
		4/21/93	48.5	55	103.5	8	55	55	55	55	55	55	55	55	55	55	51	51	51
		4/21/93	48.5	60	108.5	8	60	60	60	60	60	60	60	60	60	60	52	52	52
		4/21/93	48.5	66	114.5	8	66	66	66	66	66	66	66	66	66	66	60	60	60
		4/21/93	48.5	72	120.5	8	72	72	72	72	72	72	72	72	72	72	72	72	72
		4/21/93	48.5	74	122.5	8	74	74	74	74	74	74	74	74	74	74	74	74	74
		9/21/93	54.6	84	138.6	8	84	84	84	84	84	84	84	84	84	84	82	82	82
		9/21/93	54.6	82	136.6	8	82	82	82	82	82	82	82	82	82	82	81	81	81
		9/21/93	54.6	80	134.6	8	80	80	80	80	80	80	80	80	80	80	79	79	79
		9/21/93	54.6	79	133.6	8	79	79	79	79	79	79	79	79	79	79	78	78	78
		9/21/93	54.6	77	131.6	8	77	77	77	77	77	77	77	77	77	77	76	76	76
		9/21/93	54.6	76	130.6	8	76	76	76	76	76	76	76	76	76	76	77	77	77

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature			
							1" depth	2" depth	3" depth	4" depth
SPS-6 Bethany, MO	9 - Crack/Break & Seat	5/5/92	66.2	71	137.2	4	71			
		5/5/92	66.2	73	139.2	4	73			
		5/5/92	66.2	73	139.2	4	73			
		5/5/92	66.2	76	142.2	4	76			
		5/5/92	66.2	77	143.2	4	77			
		5/5/92	66.2	79	145.2	4	79			
		5/5/92	66.2	77	143.2	4	77			
		4/6/93	36.2	68	104.2	4	68	60		
		4/6/93	36.2	68	104.2	4	68	61		
		4/6/93	36.2	69	105.2	4	69	62		
		4/6/93	36.2	70	106.2	4	70	62		
		4/6/93	36.2	68	104.2	4	68	61		
		4/08/93	39.4	57	96.4	4	57	56		
		4/08/93	39.4	58	97.4	4	58	54		
		4/08/93	39.4	54	93.4	4	54	52		
		9/2/93	54.6	64	118.6	4	64	62		
		9/2/93	54.6	67	121.6	4	67	62		
		9/2/93	54.6	73	127.6	4	73	68		
		9/2/93	54.6	75	129.6	4	75	72		
		9/2/93	54.6	78	132.6	4	78	75		
										72

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature	1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth
SFS-6 Bethany, MO	10 - Crack/Break & Seat	5/05/92	66.2	83	149.2	8	83	84	84	85	86	86	80	80	76
		5/05/92	66.2	84	150.2	8	84	85	85	86	86	86	81	81	76
		5/05/92	66.2	85	151.2	8	85	86	86	86	86	86	81	81	75
		5/05/92	66.2	86	152.2	8	86	87	87	87	87	87	82	82	75
		5/05/92	66.2	84	150.2	8	84	85	85	85	85	85	81	81	75
		5/05/92	66.2	82	148.2	8	82	83	83	83	83	83	81	81	75
		4/09/93	42.2	41	63.2	8	41	42	42	42	42	42	42	42	44
		4/09/93	42.2	44	66.2	8	44	45	45	45	45	45	45	45	45
		4/09/93	42.2	49	91.2	8	49	49	49	49	49	49	49	49	48
		4/09/93	42.2	53	95.2	8	53	53	53	53	53	53	53	53	47
		4/09/93	42.2	63	117.3	8	63	63	63	63	63	63	63	63	47
		4/27/93	54.3	63	117.3	8	63	63	63	63	63	63	63	63	64
		4/27/93	54.3	63	116.3	8	62	62	62	62	62	62	62	62	64
		4/27/93	54.3	62	116.3	8	62	62	62	62	62	62	62	62	65
		4/27/93	54.3	65	119.3	8	65	65	65	65	65	65	65	65	65
		4/27/93	54.3	67	121.3	8	67	67	67	67	67	67	67	67	65
		4/27/93	54.3	68	122.3	8	68	68	68	68	68	68	68	68	66
		4/27/93	54.3	75	129.3	8	75	75	75	75	75	75	75	75	66
		4/27/93	54.3	72	126.3	8	72	72	72	72	72	72	72	72	67
		9/21/93	54.6	85	139.6	8	85	85	85	85	85	85	84	84	83
		9/21/93	54.6	90	144.6	8	90	90	90	90	90	90	90	90	90
		9/21/93	54.6	91	145.6	8	91	91	91	91	91	91	90	90	88
		9/21/93	54.6	88	142.6	8	88	88	88	88	88	88	87	87	87
		9/21/93	54.6	87	141.6	8	87	87	87	87	87	87	86	86	86
		9/21/93	54.6	84	138.6	8	84	84	84	84	84	84	83	83	83

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pov. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature						
							1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth
SPS-6 Bethany, MO	11 - Rubblized	6/04/92	63.1	105	168.1	.12	105	104	109	92	90	87	87
		6/04/92	63.1	104	167.1	.12	109	12	109	94	94	89	89
		6/04/92	63.1	109	172.1	.12	109	12	109	94	95	94	94
		6/04/92	63.1	109	172.1	.12	112	12	112	95	96	94	94
		6/04/92	63.1	112	175.1	.12	112	12	110	96	97	93	93
		6/04/92	63.1	110	173.1	.12	110	12	107	97	97	92	92
		6/04/92	63.1	107	170.1	.12	107	12	106	96	96	92	92
		6/04/92	63.1	106	169.1	.12	106	12	104	96	96	92	92
		6/04/92	63.1	104	167.1	.12	104	12	48	47	47	47	47
		11/11/92	37.9	48	85.9	.12	48	12	48	47	47	47	47
		11/11/92	37.9	48	85.9	.12	48	12	48	47	47	47	47
		11/11/92	37.9	48	85.9	.12	48	12	48	47	47	47	47
		11/11/92	37.9	48	85.9	.12	48	12	48	47	47	46	46
		11/11/92	37.9	48	85.9	.12	48	12	48	47	47	46	46
		11/11/92	37.9	48	85.9	.12	48	12	48	47	47	46	46
		4/29/93	58.3	76	134.3	.12	76	12	76	65	65	62	62
		4/29/93	58.3	76	134.3	.12	76	12	76	77	77	74	74
		5/10/93	65.6	83	148.6	.12	83	12	83	77	77	73	73
		5/10/93	65.6	83	148.6	.12	83	12	80	77	77	73	73
		5/10/93	65.6	80	145.6	.12	80	12	75	76	76	73	73
				75	140.6	.12							

Location	Test Section No.	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day*surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature	1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth
SPS-6 Bethany, MO	12 - Rubblized	4/29/92	44.9	57	101.9	8	57	60	62	62	60	60	60	63	63
		4/29/92	44.9	61	105.9	8	61	61	61	61	61	61	61	66	66
	5/04/92	64	97	161	8	97	97	97	97	97	97	97	97	88	88
	5/04/92	64	98	162	8	98	98	98	98	98	98	98	98	86	86
	5/04/92	64	96	160	8	96	96	96	96	96	96	96	96	86	86
	5/04/92	64	95	159	8	95	95	95	95	95	95	95	95	86	86
	5/04/92	64	93	157	8	93	93	93	93	93	93	93	93	85	85
	5/04/92	64	91	155	8	91	91	91	91	91	91	91	91	67	67
	5/05/92	66.2	60	126.2	8	60	60	60	60	60	60	60	60	64	64
	5/05/92	66.2	59	125.2	8	59	59	59	59	59	59	59	59	63	63
	5/05/92	66.2	61	127.2	8	61	61	61	61	61	61	61	61	63	63
	5/05/92	66.2	64	130.2	8	64	64	64	64	64	64	64	64	63	63
	4/21/93	48.5	72	120.5	8	72	72	72	72	72	72	72	72	61	61
	4/21/93	48.5	69	117.5	8	69	69	69	69	69	69	69	69	58	58
	4/21/93	48.5	68	116.5	8	68	68	68	68	68	68	68	68	59	59
	4/21/93	48.5	70	118.5	8	70	70	70	70	70	70	70	70	60	60
	4/21/93	48.5	71	119.5	8	71	71	71	71	71	71	71	71	63	63
	4/21/93	48.5	70	118.5	8	70	70	70	70	70	70	70	70	64	64
	4/21/93	48.5	68	116.5	8	68	68	68	68	68	68	68	68	73	73
	9/22/93	56.3	82	138.3	8	82	82	82	82	82	82	82	82	73	73
	9/22/93	56.3	83	139.3	8	83	83	83	83	83	83	83	83	75	75
	9/22/93	56.3	81	137.3	8	81	81	81	81	81	81	81	81	75	75
	9/22/93	56.3	84	140.3	8	84	84	84	84	84	84	84	84	76	76
	9/22/93	56.3	86	142.3	8	86	86	86	86	86	86	86	86	86	86
	9/22/93	56.3	87	143.3	8	87	87	87	87	87	87	87	87	87	87

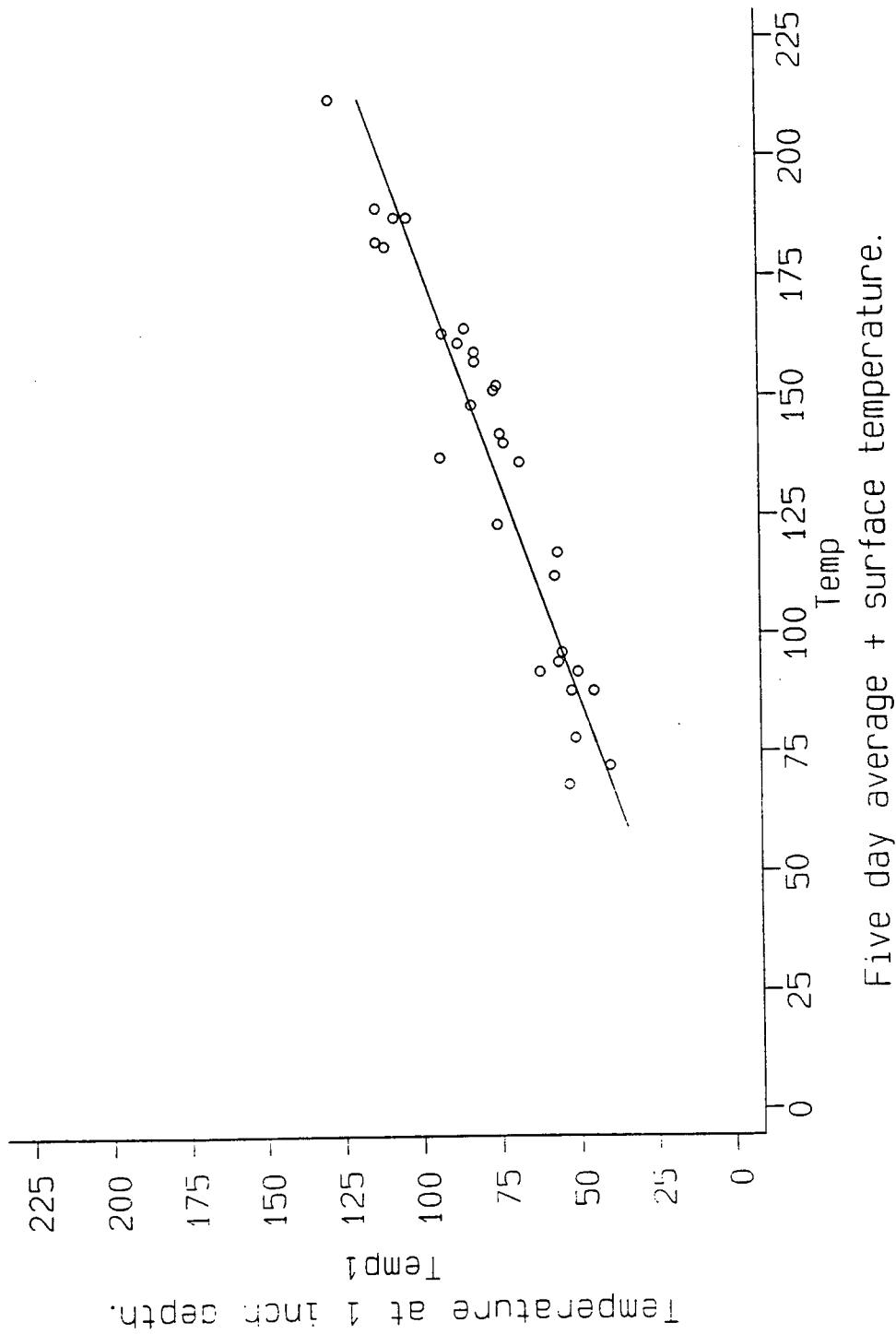
Location	Test Section No. & preparation	Date Tested	5-day avg. temperature surf. temp.	Asphalt pH 5-day+surf. temperature	AC Thickness (inches)	Actual Pavement Temperature						
						1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth
SPS-6 Bethany, MO	13 - Rubblized	6/02/92	57.3	88	145.3	12	86			83		
		6/02/92	57.3	85	142.3	12	85			80		
		6/04/92	63.1	70	133.1	12	70			70		
		6/04/92	63.1	76	139.1	12	76			72		
		6/04/92	63.1	79	142.1	12	79			72		
		6/04/92	63.1	74	137.1	12	74			73		
		6/04/92	63.1	85	148.1	12	85			74		
		6/04/92	63.1	90	153.1	12	90			77		
		6/04/92	63.1	95	158.1	12	95			80		
		6/04/92	63.1	97	160.1	12	97			79		
		6/04/92	63.1	99	162.1	12	99			80		
		6/04/92	63.1	106	169.1	12	106			82		
		11/16/92	39.2	68	107.2	12	68			87		
		11/16/92	39.2	66	105.2	12	66			83		
		11/16/92	39.2	65	104.2	12	65			80		
		11/17/92	37.9	65	104.2	12	65			80		
		11/17/92	37.9	37	74.9	12	37			80		
		11/17/92	37.9	40	77.9	12	40			80		
		11/17/92	37.9	42	79.9	12	42			87		
		11/17/92	37.9	43	80.9	12	43			83		
		4/27/93	54.3	73	127.3	12	73			56		
		4/27/93	54.3	72	126.3	12	72			55		
		4/29/93	58.3	63	121.3	12	63			61		
		4/29/93	58.3	62	120.3	12	62			60		
		4/29/93	58.3	63	121.3	12	63			61		
		4/29/93	58.3	68	126.3	12	68			61		
		4/29/93	58.3	74	132.3	12	74			63		

Location	Test Section No.	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature 1" depth	2" depth	3" depth	4" depth	5" depth	6" depth	7" depth	8" depth
SPS-6 Bethany, MO	14 - Rubblized	5/06/92	65.2	51	116.2	8	51	53	58	55	54	54	59	59
		5/06/92	65.2	53	118.2	8	53	58	58	55	58	58	62	62
		5/06/92	65.2	58	123.2	8	58	62	66	66	60	60	57	57
		5/06/92	65.2	57	122.2	8	57	62	68	68	61	61	60	59
		5/06/92	65.2	62	127.2	8	62	66	71	71	77	77	79	79
		5/06/92	65.2	66	131.2	8	66	71	76	76	81	81	81	81
		6/02/92	57.3	88	145.3	8	88	88	88	88	88	88	88	88
		6/02/92	57.3	88	145.3	8	88	88	88	88	88	88	88	88
		6/02/92	57.3	83	140.3	8	83	83	83	83	83	83	83	83
		6/02/92	57.3	83	140.3	8	83	83	83	83	83	83	83	83
		11/05/92	41.4	36	77.4	8	36	36	36	36	37	37	39	39
		11/05/92	41.4	35	76.4	8	35	35	35	35	37	37	38	38
		11/05/92	41.4	35	76.4	8	35	35	35	35	37	37	38	38
		11/05/92	41.4	34	75.4	8	34	34	34	34	37	37	38	38
		4/21/93	48.5	71	119.5	8	71	71	71	71	69	69	66	66
		4/21/93	48.5	73	121.5	8	73	73	73	73	72	72	67	67
		4/27/93	54.3	72	126.3	8	72	72	72	72	70	70	69	69
		4/27/93	54.3	72	126.3	8	72	72	72	72	70	70	69	69
		4/27/93	54.3	73	127.3	8	73	73	73	73	71	71	70	70
		4/27/93	54.3	73	127.3	8	73	73	73	73	72	72	70	70
		9/22/93	56.3	85	141.3	8	85	85	85	85	83	83	75	75
		9/22/93	56.3	83	139.3	8	83	83	83	83	82	82	82	82

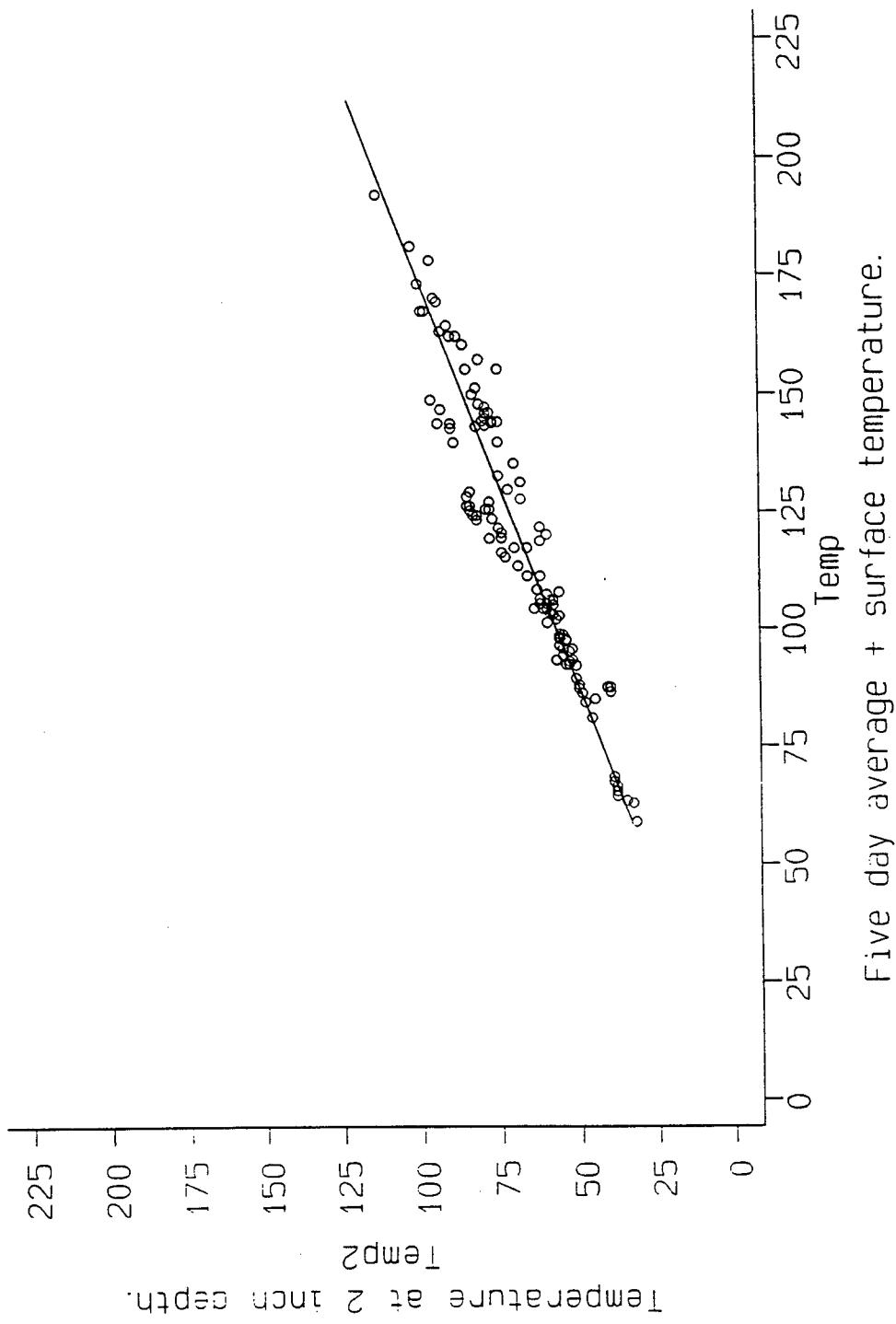
Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Asphalt pvt. surf. temp.	5-day+surf. temperature	AC Thickns. (inches)	Actual Pavement Temperature				
							1" depth	2" depth	3" depth	4" depth	5" depth
SPS-6 Bethany, MO	15 - Typical Project	4/23/92	47.2	56	103.2	5	56				55
		4/23/92	47.2	61	108.2	5	61				57
		4/23/92	47.2	59	106.2	5	59				57
		4/23/92	47.2	57	104.2	5	57				56
		4/23/92	47.2	58	105.2	5	58				55
		4/23/92	47.2	58	105.2	5	58				55
		4/23/92	47.2	60	107.2	5	60				56
		4/23/92	47.2	62	109.2	5	62				57
		4/23/92	47.2	62	109.2	5	62				57
		4/23/92	47.2	71	118.2	5	71				62
		4/23/92	47.2	69	116.2	5	69				60
		4/23/93	50.3	59	109.3	5	59				59
		4/23/93	50.3	63	113.3	5	63				61
		5/13/93	64.6	58	122.6	5	58				58
		5/13/93	64.6	59	123.6	5	59				59
		5/13/93	64.6	60	124.6	5	60				58
		5/13/93	64.6	62	126.6	5	62				59
		5/13/93	64.6	70	134.6	5	70				61
		5/13/93	64.6	73	137.6	5	73				66
		5/13/93	64.6	78	142.6	5	78				69

Location	Test Section No. & preparation	Date Tested	5-day avg. temperature	Concrete pvt. surf. temp.	5-day surf. temperature	AC Thickns. (Inches)	Actual Pavement Temperature				
							1" depth	2" depth	3" depth	4" depth	5" depth
SFS-6 Bethany, MO	16 - Min. Restoration	4/22/92	51.3	58	109.3	8	58				51
		4/22/92	51.3	63	114.3	8	63				58
		4/22/92	51.3	70	121.3	8	70				58
		4/22/92	51.3	71	122.3	8	71				59
		4/22/92	51.3	72	123.3	8	72				64
		4/22/92	51.3	78	129.3	8	78				66
		4/22/92	51.3	74	125.3	8	74				63
		4/22/92	51.3	74	125.3	8	74				64
		4/22/92	51.3	79	130.3	8	79				68
		4/22/92	51.3	73	124.3	8	73				66
		4/22/92	51.3	73	124.3	8	73				66
		4/22/92	50.3	50	100.3	8	50				59
		4/23/93	50.3	52	102.3	8	52				59
		4/23/93	50.3	95	160.2	8	95				90
		5/12/93	65.2	93	158.2	8	93				89
		5/12/93	65.2	90	155.2	8	90				88
		5/12/93	65.2	90	155.2	8	90				89
		5/12/93	65.2	87	152.2	8	87				86
		5/12/93	65.2	87	152.2	8	87				85
											81

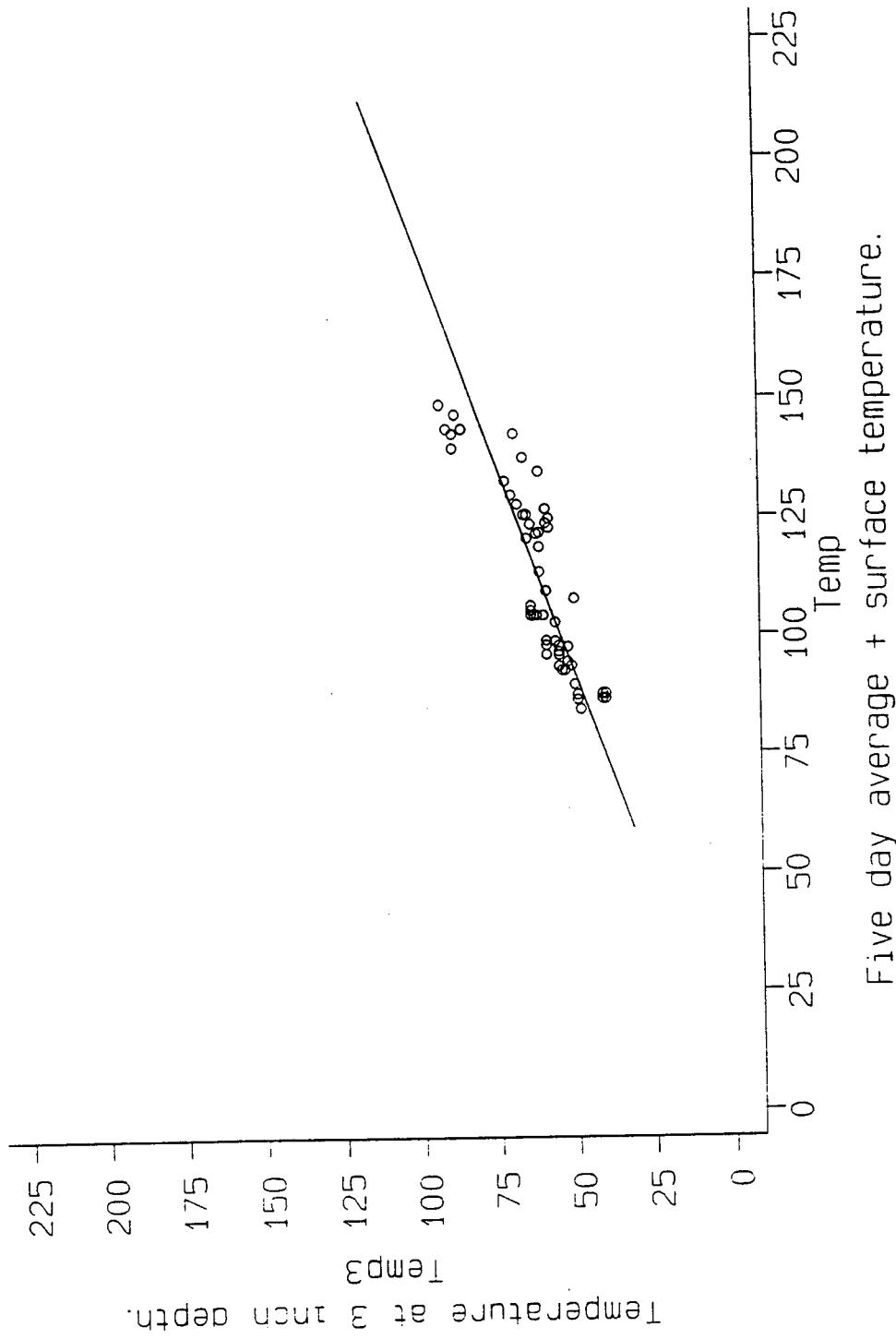
Five day average + Surface temp vs. temperature at 1 inch depth.
Regression Equation: Temp1=5.49*Temp+2.152, R²=.8966, n=30, S(y,x)=7.6



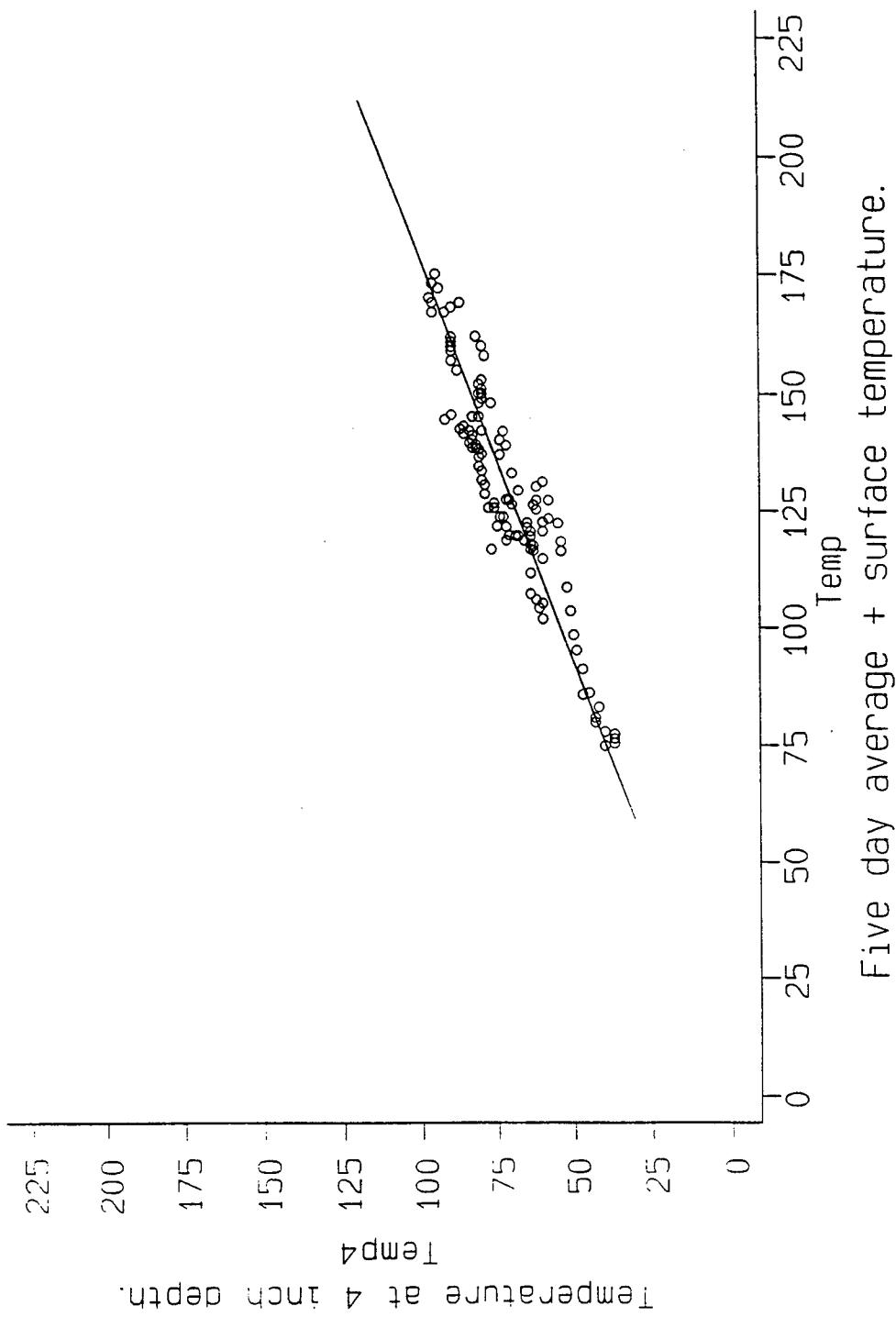
Five day average + surface temp vs. temperature at 2 inch depth.
Regression Equation: Temp2=.577*Temp-.482, R²=.9070, n=132, S(y,x)=5.5



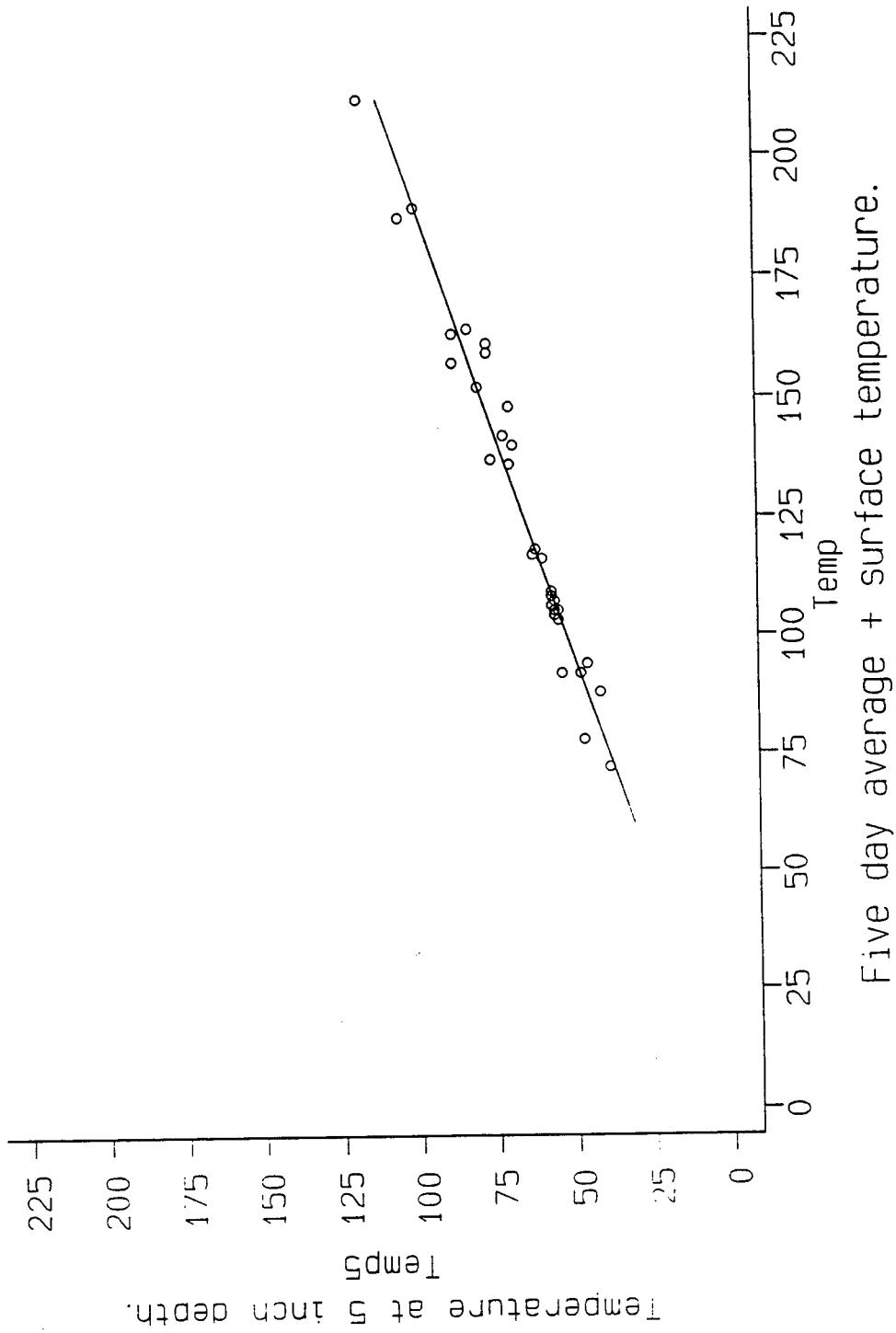
Five day average + surface temp vs. temperature at 3 inch depth.
Regression Equation: Temp3=.564*Temp-1.88, R²=.7514, n=56, S(y,x)=6.3



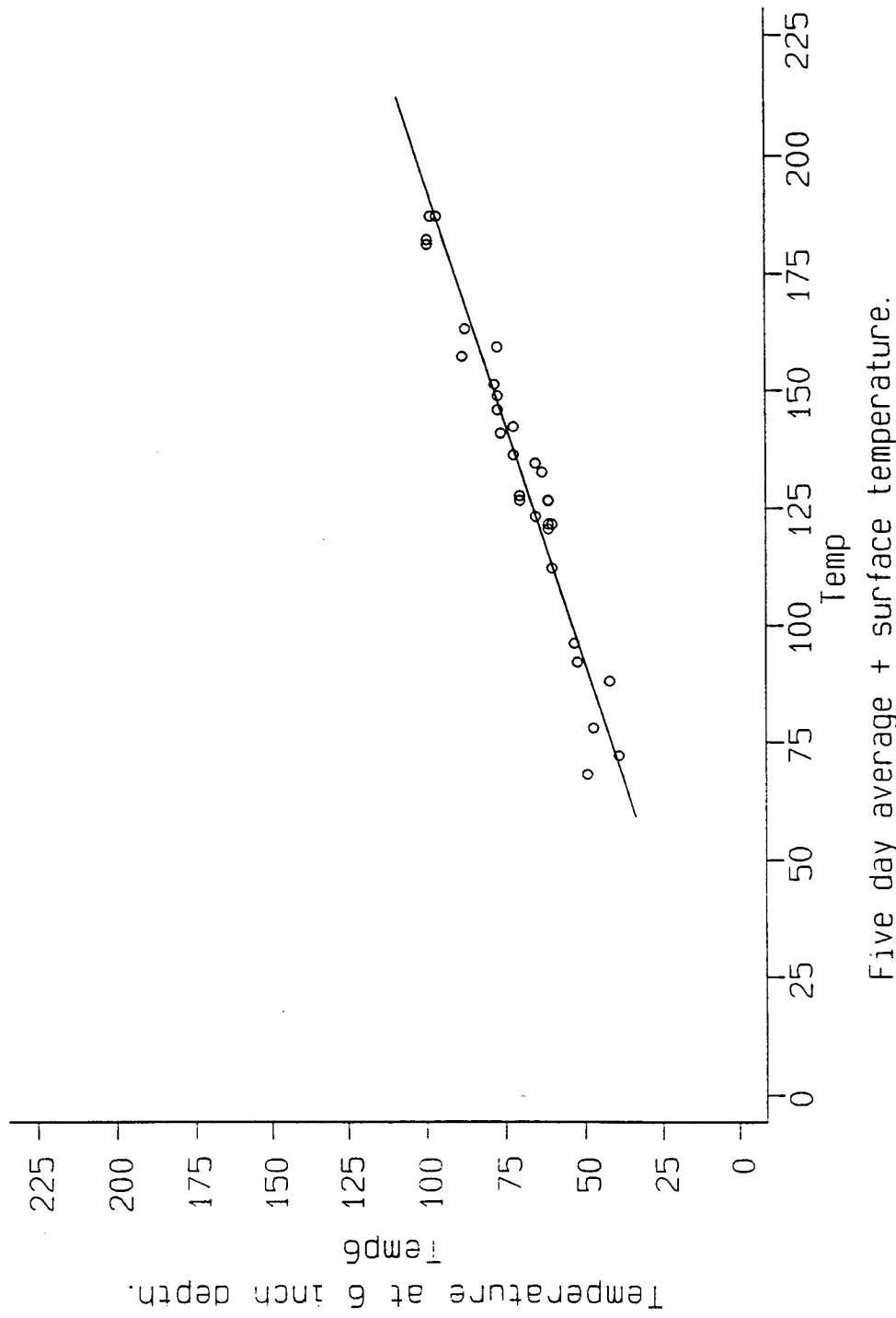
Five day average + surface temp vs. temperature at 4 inch depth.
Regression Equation: Temp4=.583*Temp-4.025, R²=.8851, n=125, S(y,x)=5.3



Five day average + surface temp vs. temperature at 5 inch depth.
Regression Equation: Temp5 = .528 * Temp - .363, R square=.9580, n=32, S(y,x)=3.9

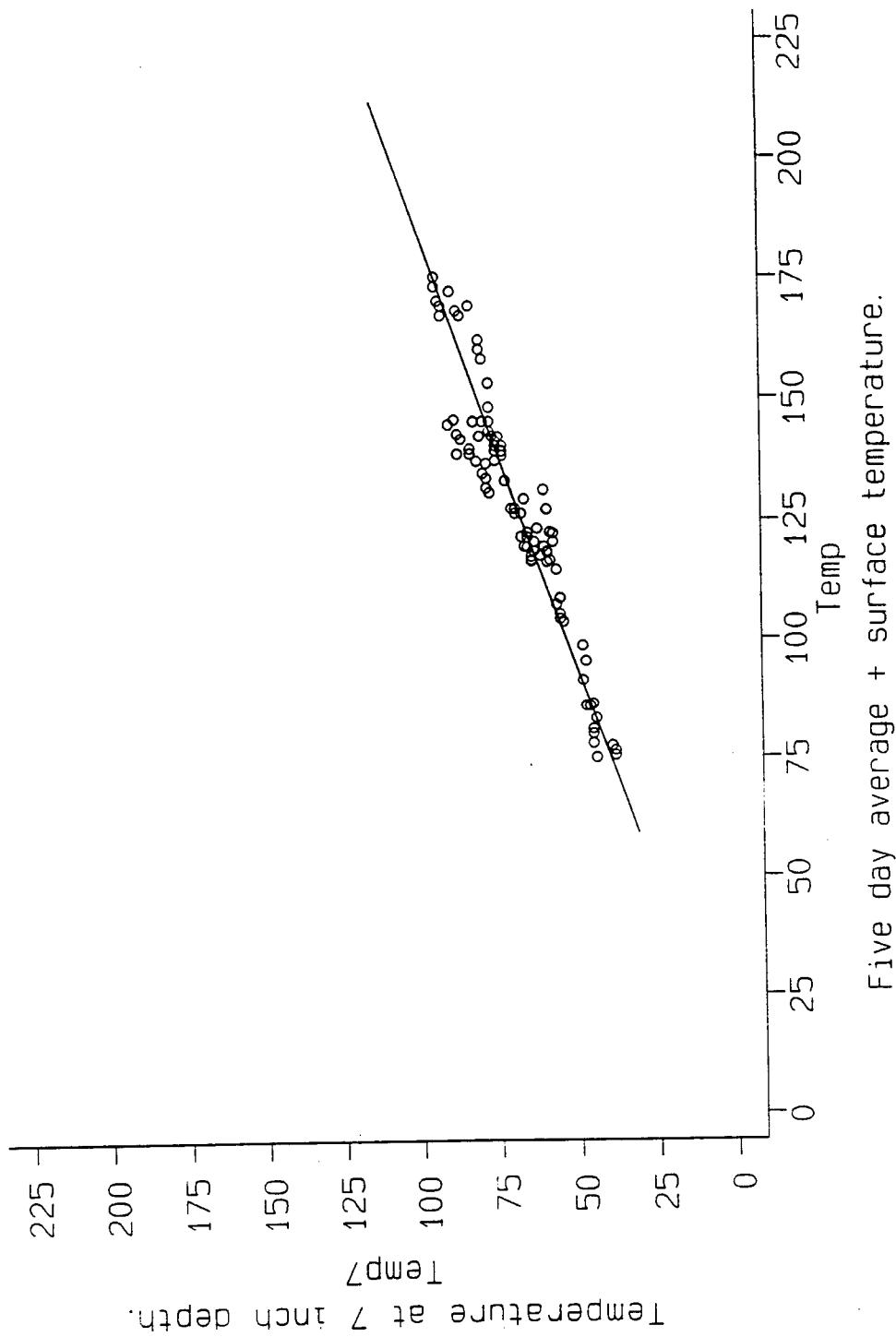


Five day average + surface temp vs. temperature at 6 inch depth.
Regression Equation: Temp6=.490*Temp+4.725, R²=.9347, n=31, S(y,x)=4.2

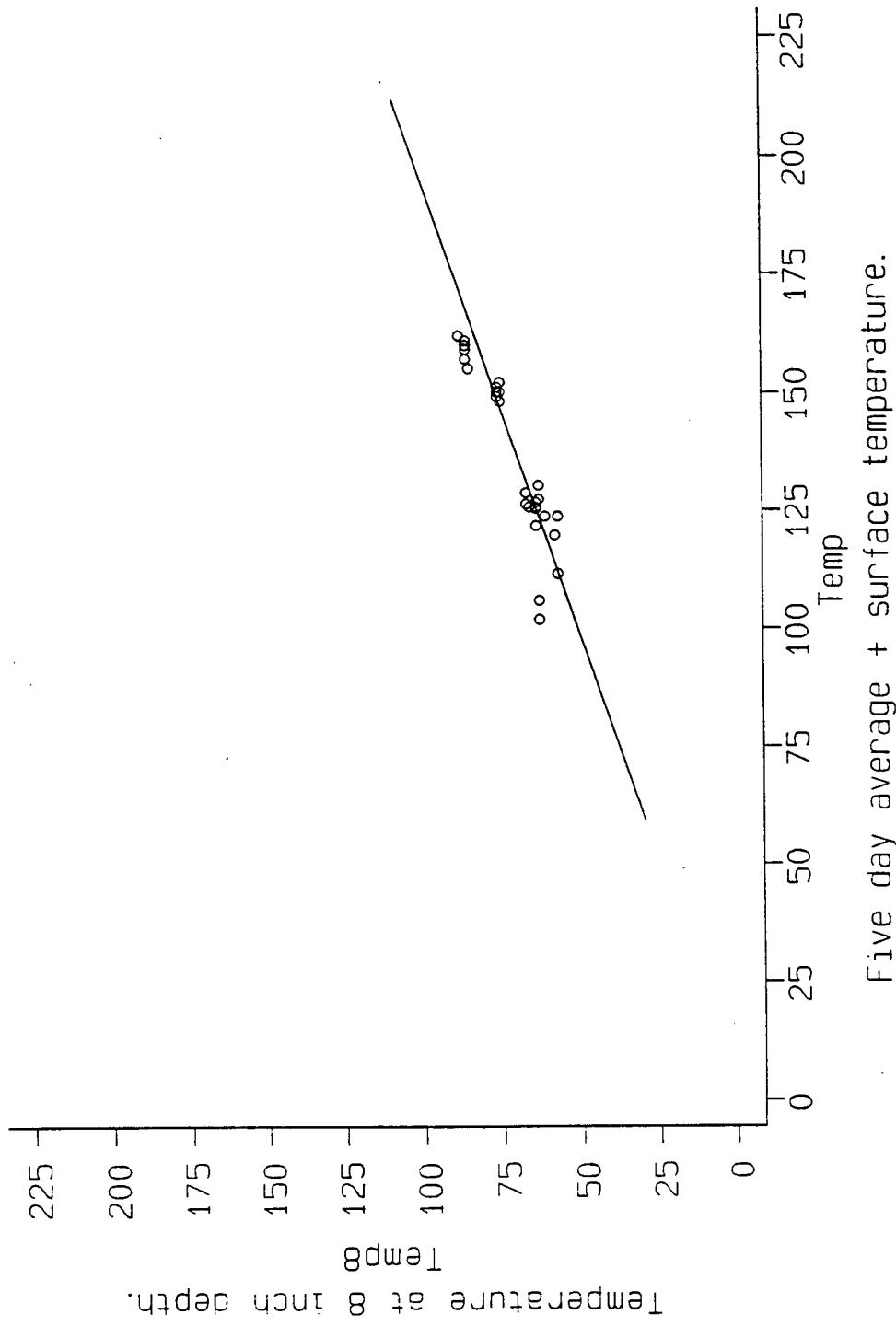


Five day average + surface temperature.

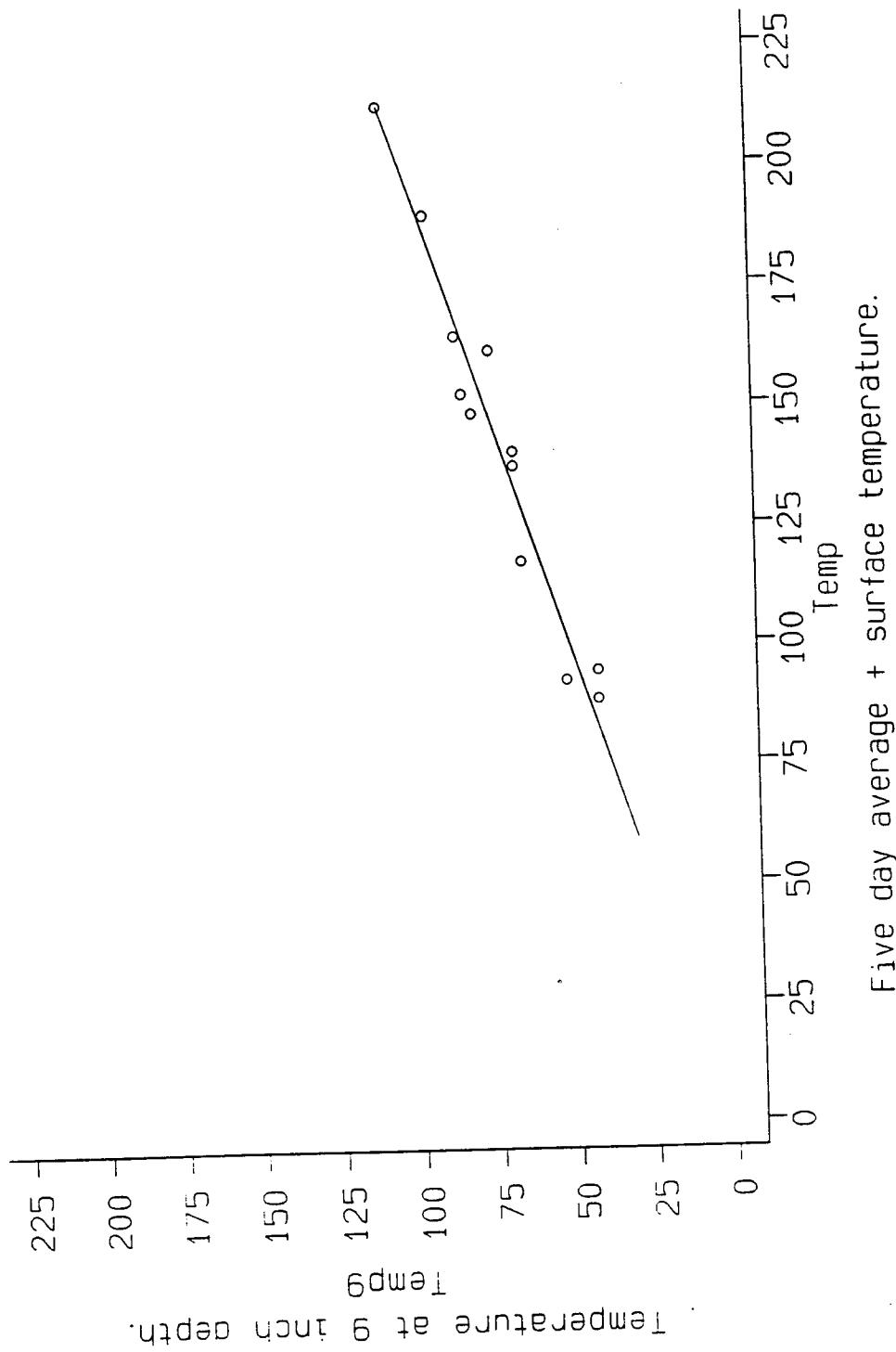
Five day average + surface temp vs. temperature at 7 inch depth.
Regression Equation: Temp_y = .545x Temp - 1.228, R²=.9093, n=98, S(y,x)=4.6



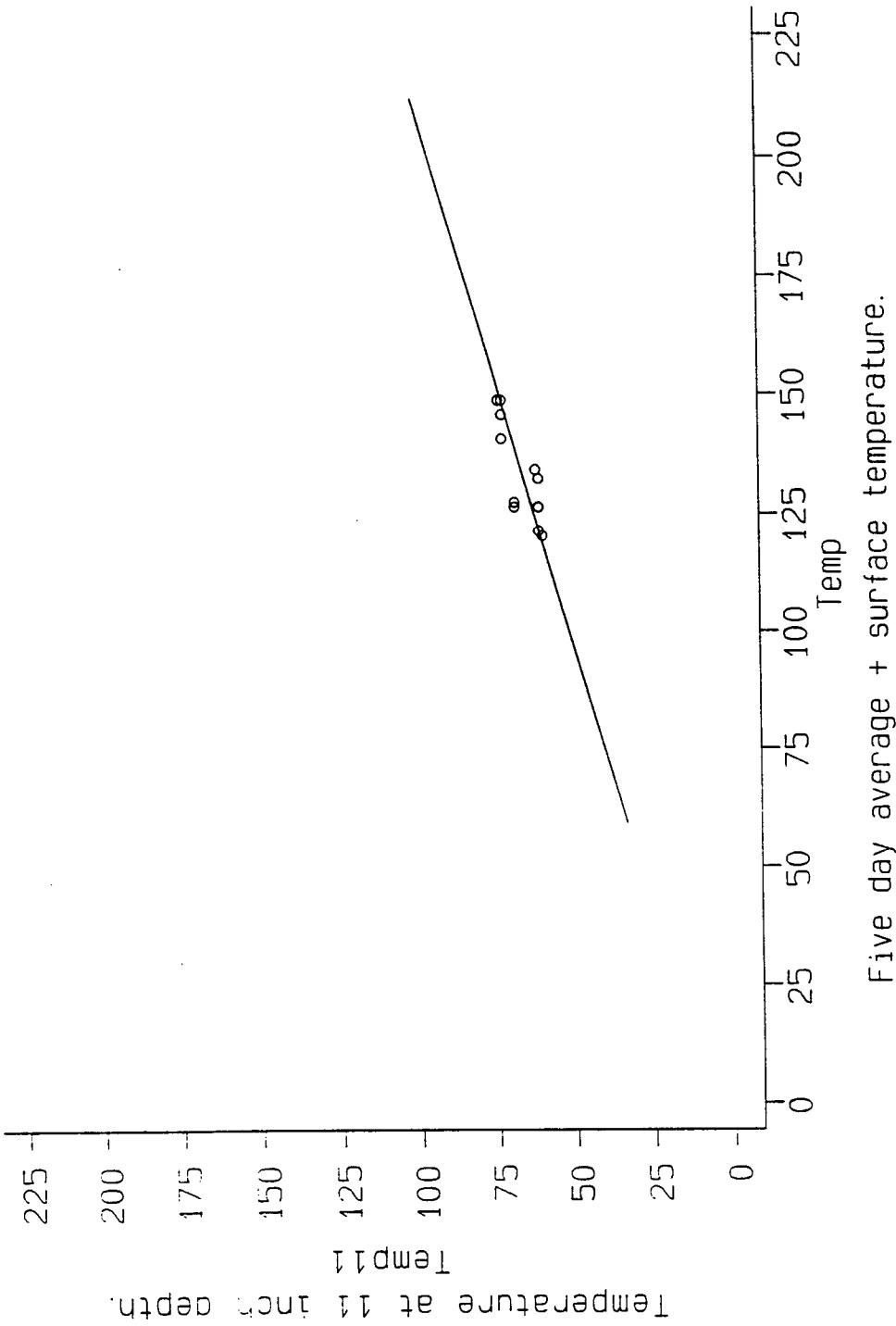
Five day average + surface temp vs. temperature at 8 inch depth.
Regression Equation: Temp8=.515*Temp-.632, R square=.8476, n=27, S(y,x)=4.0



Five day average + Surface temp vs. temperature at 9 inch depth.
Regression Equation: Temp9=.514*Temp-.167, R²=.9447, n=12, S(y,x)=5.0



Five day average + surface temp vs. temperature at 11 inch depth.
Regression Equation: Temp₁₁=.441*Temp+7.420, R²=.6208, n=13, S(y,x)=3.7



Five day average + surface temperature.

Regression values for 5 day average + surface temperature vs. depth temperature

(Asphalt pavement temperatures.)

Estimated linear regression equation: $\hat{Y} = m \cdot X + b$

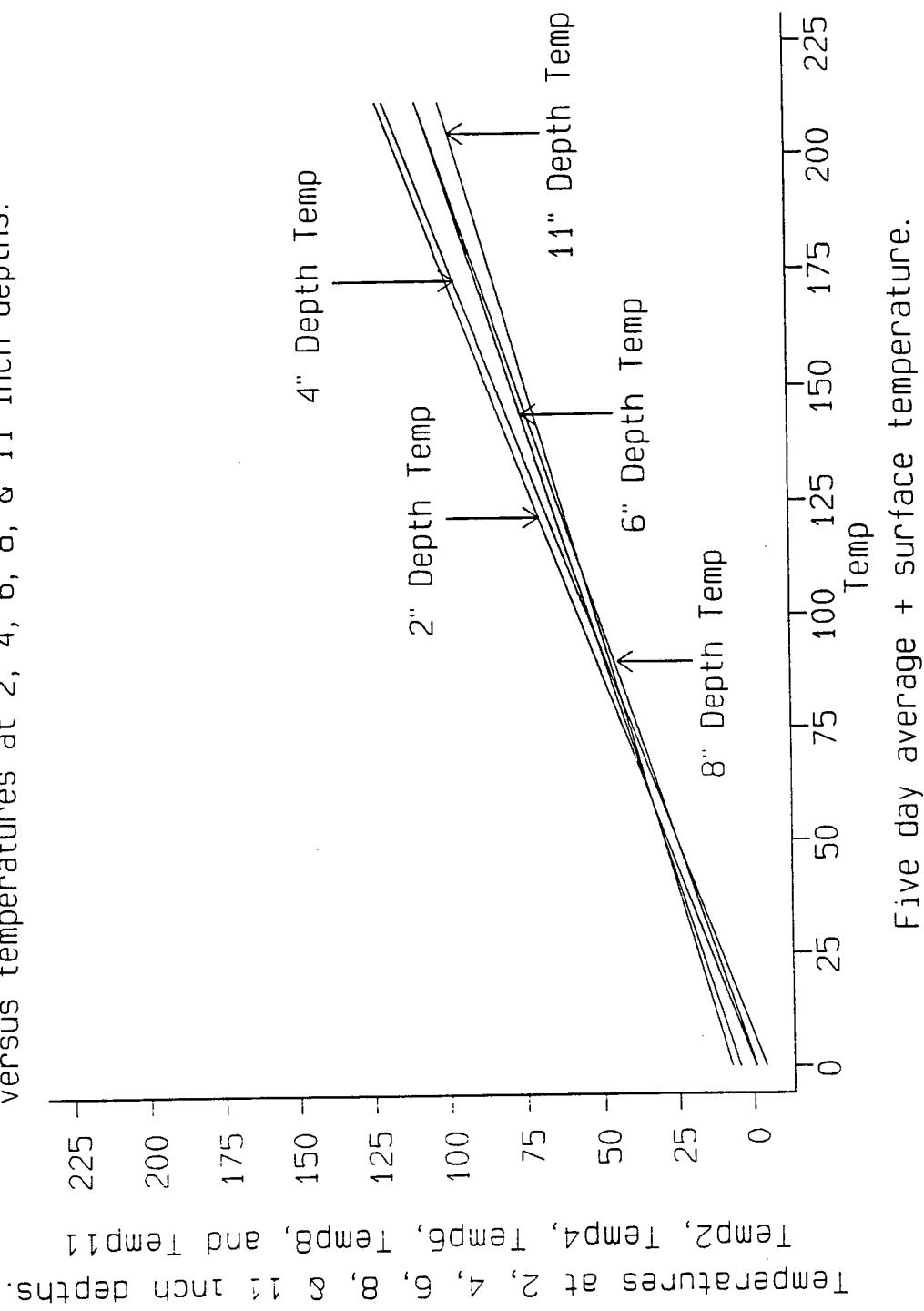
X = 5 Day average + surface temperature

Y = Depth temperature

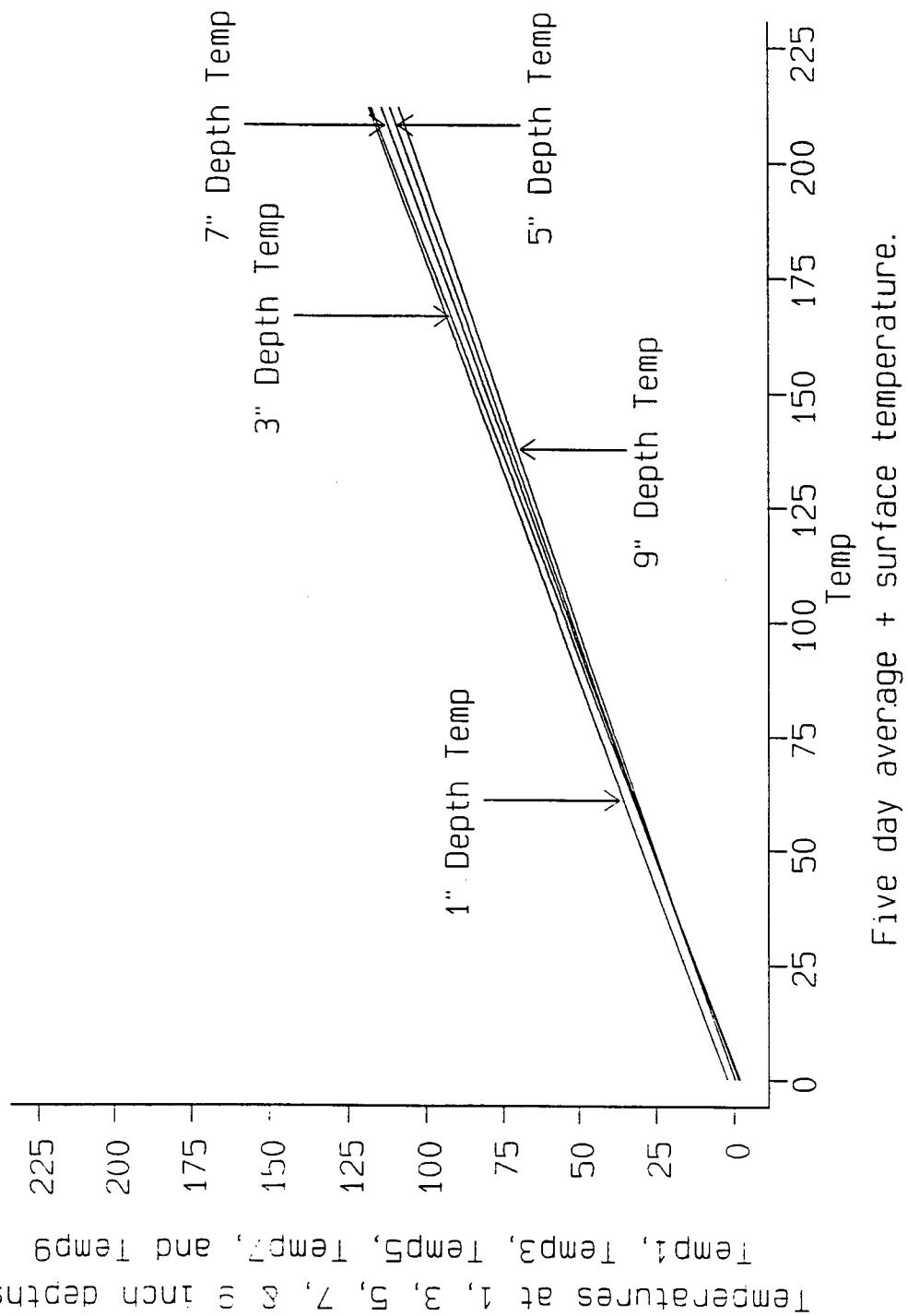
Variable:	Sample Size n:	Mean					
		F-value:	Prob. > F:	Significant (Y/N):	Rsquare:	MSE:	SqRt(MSE)
Temp1	30	242.68	0	Y	0.8966	57.95516	7.612829
Temp2	132	1267.5	0	Y	0.907	30.62344	5.533845
Temp3	56	163.22	0	Y	0.7514	40.06890	6.330000
Temp4	125	947.5	0	Y	0.8851	28.48441	5.337078
Temp5	32	683.99	0	Y	0.958	14.97158	3.869313
Temp6	31	414.9	0	Y	0.9347	17.90409	4.231323
Temp7	98	962.84	0	Y	0.9093	21.13139	4.596889
Temp8	27	139.09	0	Y	0.8476	16.31185	4.038794
Temp9	12	170.74	0	Y	0.9447	25.42144	5.041968
Temp11	13	18.01	0.0014	Y	0.6208	13.68139	3.698836

Variable:	b	m	Std. Err.	Significant		
	Y-int.:	Slope:	Coeff.(m):	T-value:	Prob. > T:	(Y/N):
Temp1	2.151643	0.548537	0.035211	15.57827	0	Y
Temp2	-0.48153	0.577301	0.016215	35.60181	0	Y
Temp3	-1.88034	0.563716	0.044124	12.77571	0	Y
emp4	-4.02543	0.582710	0.018930	30.78141	0	Y
Temp5	-0.36269	0.528364	0.020202	26.15316	0	Y
Temp6	4.724505	0.490000	0.024056	20.36915	0	Y
Temp7	-1.22778	0.545164	0.017569	31.02954	0	Y
Temp8	-0.63232	0.514906	0.043659	11.79378	0	Y
Temp9	-0.16731	0.513581	0.039304	13.06674	0	Y
Temp11	7.420474	0.441511	0.104030	4.244042	0.001	Y

Regression lines for five day average + surface temperature
versus temperatures at 2, 4, 6, 8, & 11 inch depths.



Regression lines for five day average + surface temperature
versus temperatures at 1, 3, 5, 7, & 9 inch depths.

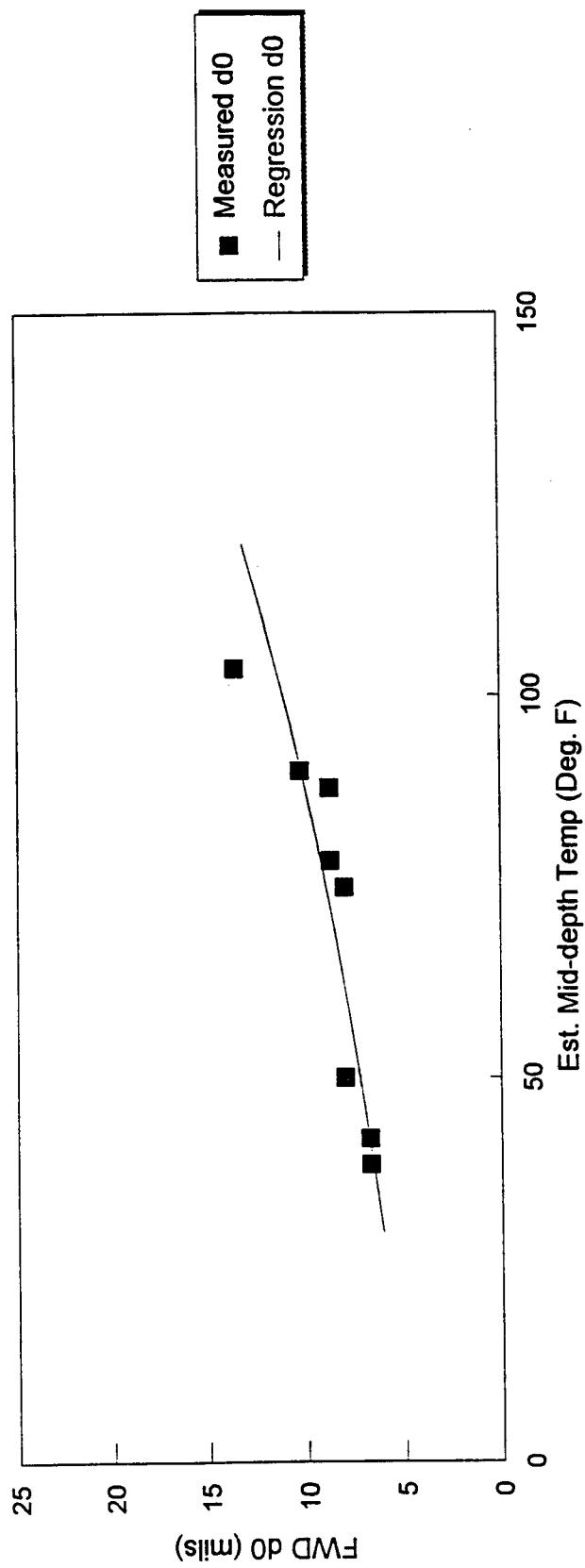


APPENDIX B

Data and Graphs Relating to the Empirical Temperature Adjustment Factor Curves

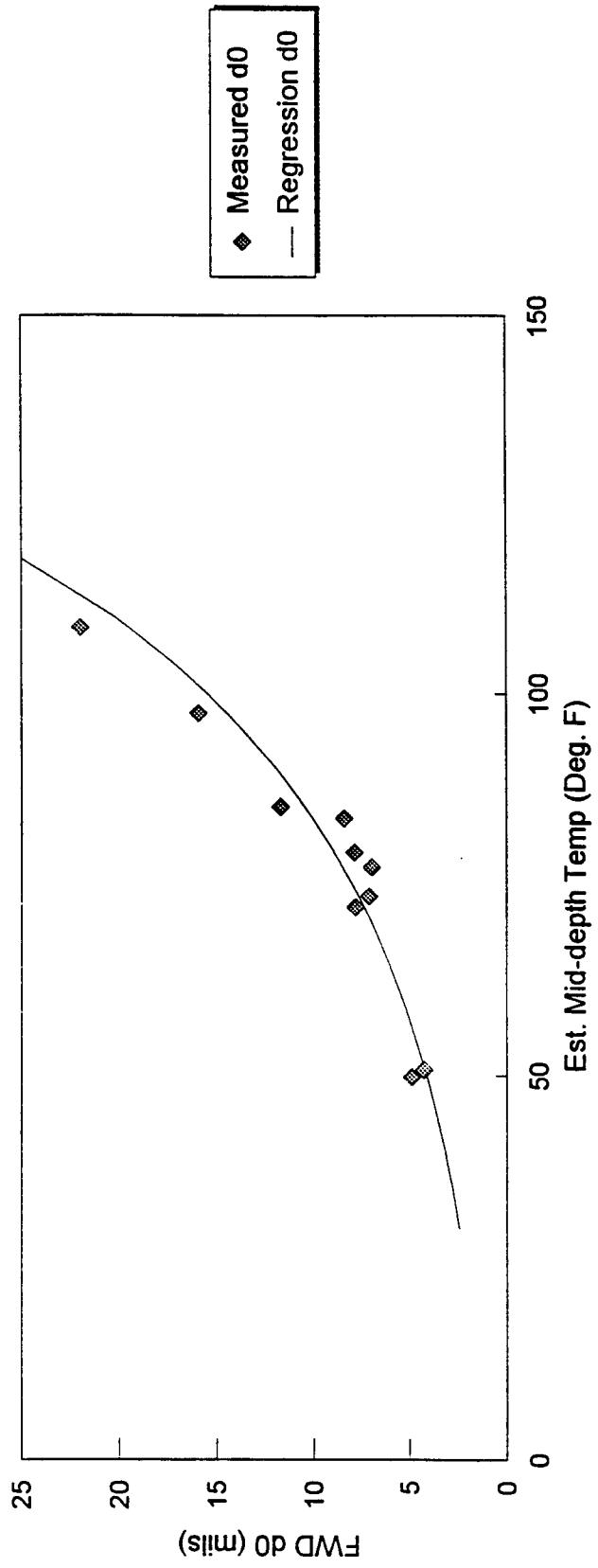
FWD Deflection Data

MT1



Normalized to 9 kip equivalent load
Plate radius = 5.9"

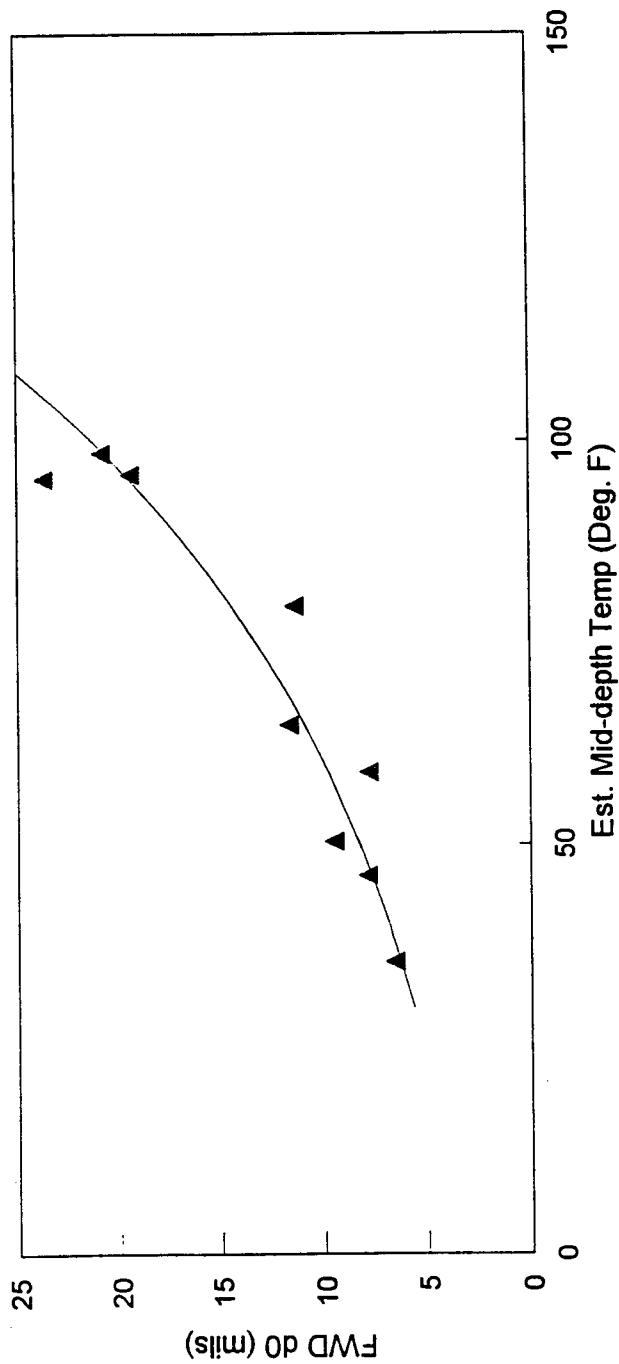
FWD Deflection Data
MT2



Normalized to 9 kip equivalent load
Plate radius = 5.9"

FWD Deflection Data

MT3



Normalized to 9 kip equivalent load
Plate radius = 5.9"

MT1 Regression FWD Deflection Equation
Correlation Coefficient = 0.895540

$$d_0 = 4.738428e^{(0.008485 * \text{TEMP})}$$

MT2 Regression FWD Deflection Equation
Correlation Coefficient = 0.962902

$$d_0 = 1.090210e^{(0.026551 * \text{TEMP})}$$

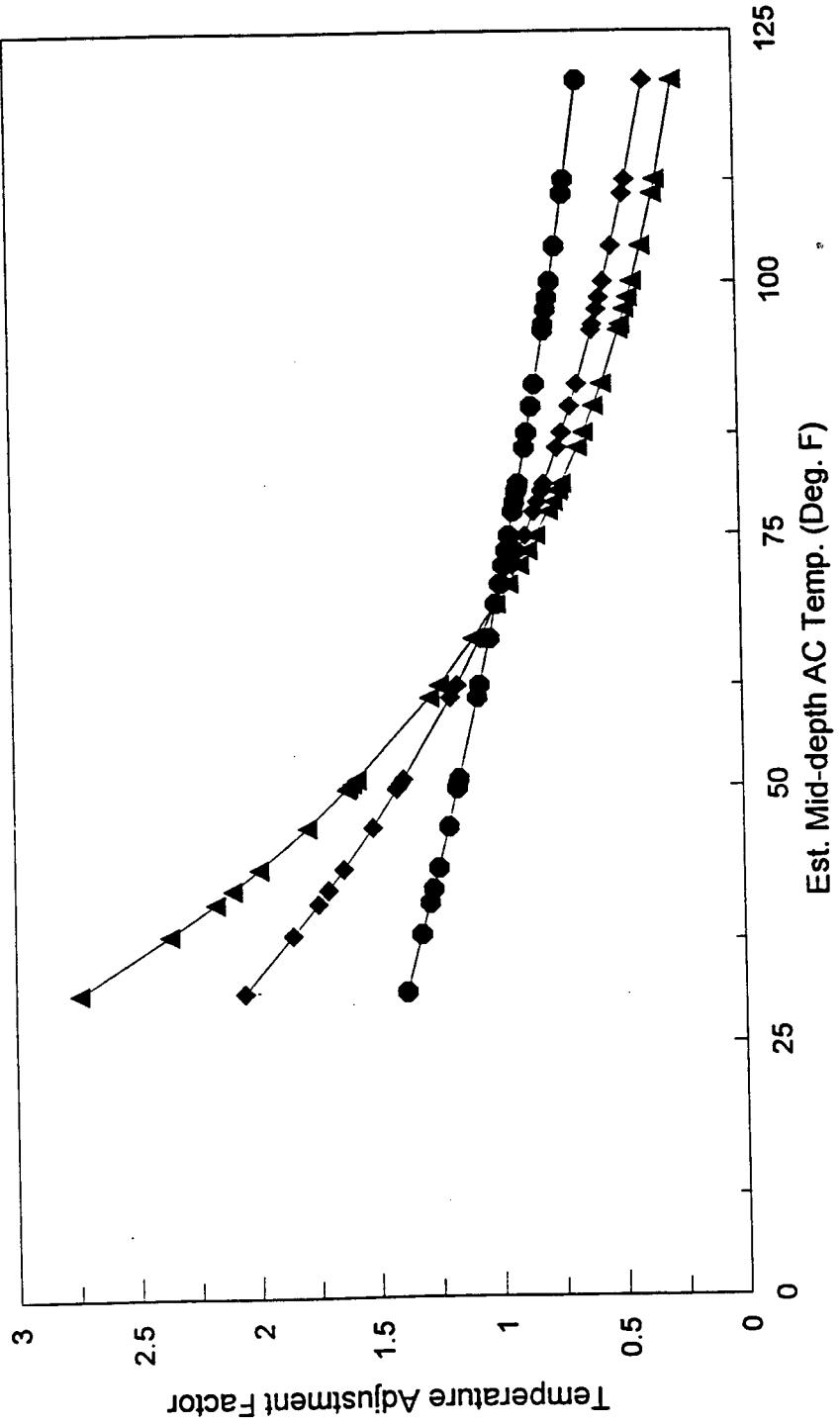
MT3 Regression FWD Deflection Equation
Correlation Coefficient = 0.950967

$$d_0 = 3.188715e^{(0.018935 * \text{TEMP})}$$

Est. mid-depth temp. (Deg. F)	FWD d0 (mils) MT1	FWD d0 (mils) MT2	FWD d0 (mils) MT3	Regr. d0 (mils) MT1	Regr. d0 (mils) MT2	Regr. d0 (mils) MT3	TAF		TAF	
							MT1	7" AC depth	MT2	10" AC depth
30	35.6	6.73	6.54	6.11	2.42	5.64	1.3805	2.7427	2.0574	2.0574
38.7	49.8	8	4.89	6.41	2.80	6.26	1.3168	2.3659	1.8511	1.8511
40	42.1	6.74	7.87	6.58	3.05	6.65	1.2822	2.1766	1.7439	1.7439
46.1	50.0	50.4	9.49	6.65	3.15	6.81	1.2682	2.1031	1.7016	1.7016
49.8	58.8	50.8	4.25	6.77	3.33	7.09	1.2459	1.9896	1.6354	1.6354
58.8	60	64.6	7.77	7.23	4.09	8.21	1.1669	1.6211	1.4126	1.4126
68	70	71.9	8.20	7.29	4.20	8.24	1.1652	1.6135	1.4079	1.4079
73.3	74.8	78.2	7.08	7.80	5.20	9.74	1.0811	1.2762	1.1905	1.1905
8.01	77.2	79.2	6.96	7.88	5.36	9.96	1.0702	1.2367	1.1640	1.1640
8.01	79.4	80	8.72	8.20	6.06	10.88	1.0290	1.0937	1.0661	1.0661
8.01	83.6	83.6	7.84	8.83	6.44	6.63	11.60	1.0000	1.0000	1.0000
8.01	85.1	85.1	11.35	8.94	7.94	13.19	0.9441	0.8351	0.8791	0.8791
8.01	87.8	87.8	7.84	9.13	8.48	13.82	0.9246	0.7824	0.8390	0.8390
8.01	90.0	90.0	8.72	9.20	8.69	14.06	0.9173	0.7634	0.8245	0.8245
8.01	95.3	95.3	7.84	9.28	8.93	14.34	0.9093	0.7427	0.8084	0.8084
8.01	95.8	95.8	11.35	9.30	8.98	14.40	0.9076	0.7384	0.8051	0.8051
8.01	97.3	97.3	8.73	9.34	9.12	14.56	0.9032	0.7272	0.7963	0.7963
8.01	98.4	98.4	10.26	9.63	10.04	15.60	0.8759	0.6606	0.7435	0.7435
8.01	100	100	10.26	9.75	10.44	16.04	0.8651	0.6353	0.7230	0.7230
8.01	103.5	103.5	10.26	9.98	11.20	16.87	0.8457	0.5919	0.6873	0.6873
8.01	108.6	108.6	10.26	10.17	11.89	17.61	0.8297	0.5575	0.6585	0.6585
8.01	110	110	10.26	10.63	13.68	19.46	0.7934	0.4848	0.5959	0.5959
8.01	120	120	10.26	11.07	15.51	21.29	0.7622	0.4276	0.5447	0.5447
8.01	13.53	13.53	20.81	10.92	14.88	20.67	0.7724	0.4456	0.5611	0.5611
8.01	21.97	21.97	13.12	12.05	20.23	25.74	0.7002	0.3279	0.4505	0.4505
8.01						31.12	0.6433	0.2514	0.3726	0.3726

Empirical Temperature Adjustment Factor Curves

MT1-MT3



APPENDIX C

Data and Graphs Relating to the Theoretical Temperature Adjustment Factor Curves

SAMPLE CALCULATIONS

Given:

h_b = 4 inches

E_b = 30 ksi

h_{ac} = 2 inches

M_r = 10 ksi

t_p = 30 deg. F

E_{ac} = 2293.46 ksi

where:

h_b = height of base

E_b = Modulus of Elasticity of base

h_{ac} = height of AC

M_r = resilient Modulus of subgrade

t_p = average AC mix temperature

E_{ac} = estimated Modulus of
Elasticity at given average
AC mix temperature

$$h_{equivalent} = h_b * \sqrt[3]{\frac{E_b}{E_{ac}}} + h_{ac}$$

$$h_{equivalent} = 4 * \sqrt[3]{\frac{30}{2293.46}} + 2 = h_{E_p} = 2.942$$

$$d_0 = 1.5pa \left\{ \frac{1}{M_r \sqrt{1 + \left(\frac{D}{a} \sqrt[3]{\frac{E_p}{M_r}} \right)^2}} + \frac{\left[1 - \frac{1}{\sqrt{1 + \left(\frac{D}{a} \right)^2}} \right]}{E_p} \right\}$$

$$d_0 = 1.5 * 82.3 * 5.9 \left\{ \frac{1}{10 \sqrt{1 + \left(\frac{2.942}{5.9} \sqrt[3]{\frac{2293.46}{10}} \right)^2}} + \frac{\left[1 - \frac{1}{\sqrt{1 + \left(\frac{2.942}{5.9} \right)^2}} \right]}{2293.46} \right\} = 22.6731 \text{ mils}$$

at $68^\circ F$, $d_0 = 27.9141 \text{ mils}$ found by the same method as above

$$TAF = \frac{d_{0_{68}}}{d_{0_T}} \text{ where } T = 30^\circ F \text{ as given above}$$

$$TAF = \frac{27.9141}{22.6731} = 1.231$$

$E_b =$	30 ksi
$h_{ac} =$	2 inches

E_b = 30 ksi
hac = 4 inches

E _a (ksi)	hEP (inches)	t _p (deg. F.)	M ₁			M ₂			M ₃			M ₄			M ₅			M ₆		
			M ₁₁ (ksi)	d ₀ (mils)	d ₁₅ (mils)	M ₁₂ (ksi)	d ₀ (mils)	d ₁₅ (mils)	M ₁₃ (ksi)	d ₀ (mils)	d ₁₅ (mils)	M ₁₄ (ksi)	d ₀ (mils)	d ₁₅ (mils)	M ₁₅ (ksi)	d ₀ (mils)	d ₁₅ (mils)	M ₁₆ (ksi)	d ₀ (mils)	d ₁₅ (mils)
2293.46	4.942	30	13.9415	10.5794	8.6896	7.4545	6.5736	5.9082	1.311	1.306	1.302	1.298	1.266	1.263	1.294	1.298	1.294	1.298	1.266	1.263
2074.71	4.974	35	14.3079	10.8542	8.9130	7.6444	6.7396	6.0562	1.282	1.277	1.273	1.269	1.231	1.228	1.234	1.237	1.234	1.231	1.228	1.231
1850.36	5.012	40	14.7340	11.1734	9.1723	7.8646	6.9319	6.2275	1.245	1.241	1.241	1.237	1.196	1.193	1.198	1.196	1.193	1.196	1.193	1.191
1627.24	5.057	45	15.2221	11.5387	9.4687	8.1160	7.1514	6.4229	1.205	1.202	1.202	1.202	1.160	1.157	1.157	1.155	1.157	1.155	1.153	1.151
1411.23	5.108	50	15.7750	11.9519	9.8035	8.3998	7.3988	6.6429	1.163	1.163	1.163	1.163	1.119	1.117	1.117	1.115	1.117	1.115	1.113	1.110
1207.09	5.167	55	16.3954	12.4149	10.1782	8.7169	7.6750	6.8883	1.119	1.119	1.119	1.119	1.113	1.112	1.112	1.112	1.113	1.112	1.112	1.110
1018.40	5.235	60	17.0863	12.9294	10.5939	9.0683	7.9806	7.1595	1.074	1.074	1.072	1.072	1.071	1.071	1.071	1.072	1.071	1.071	1.069	1.068
847.58	5.313	65	17.8505	13.4973	11.0519	9.4547	8.3163	7.4569	1.028	1.028	1.027	1.027	1.027	1.027	1.027	1.028	1.027	1.027	1.026	1.026
754.20	5.365	68	18.3455	13.8645	11.3475	9.7038	8.5323	7.6481	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
695.91	5.403	70	18.6905	14.1201	11.5531	9.8768	8.6822	7.7807	0.982	0.982	0.982	0.982	0.982	0.982	0.982	0.982	0.982	0.983	0.983	0.983
563.73	5.505	75	19.6086	14.7988	12.0980	10.3347	9.0784	8.1306	0.936	0.936	0.937	0.937	0.938	0.938	0.939	0.938	0.939	0.940	0.940	0.941
450.57	5.621	80	20.6062	15.5339	12.6865	10.8281	9.5045	8.5060	0.890	0.893	0.894	0.894	0.896	0.896	0.898	0.896	0.898	0.899	0.898	0.899
355.36	5.755	85	21.6839	16.3251	13.3180	11.3561	9.9592	8.9059	0.846	0.849	0.849	0.849	0.852	0.852	0.855	0.852	0.855	0.857	0.859	0.859
276.56	5.908	90	22.8410	17.1712	13.9910	11.9170	10.4409	9.3285	0.803	0.803	0.807	0.807	0.811	0.811	0.817	0.811	0.817	0.820	0.820	0.820
212.41	6.083	95	24.0754	18.0696	14.7027	12.5082	10.9472	9.7713	0.762	0.767	0.772	0.772	0.776	0.776	0.779	0.776	0.779	0.779	0.779	0.783
161.00	6.285	100	25.3832	19.0165	15.4497	13.1263	11.4748	10.2314	0.723	0.729	0.734	0.734	0.739	0.739	0.744	0.739	0.744	0.744	0.748	0.748
120.45	6.517	105	26.7583	20.0064	16.2269	13.7669	12.0195	10.7049	0.686	0.693	0.699	0.699	0.705	0.705	0.710	0.705	0.710	0.714	0.714	0.714
88.94	6.784	110	28.1926	21.0326	17.0285	14.4247	12.5767	11.1876	0.651	0.659	0.666	0.666	0.673	0.673	0.678	0.673	0.678	0.684	0.684	0.684
64.82	7.094	115	29.6758	22.0867	17.8473	15.0934	13.1408	11.6746	0.618	0.628	0.636	0.636	0.643	0.643	0.649	0.643	0.649	0.655	0.655	0.655
46.63	7.453	120	31.1954	23.1589	18.6753	15.7664	13.7063	12.1609	0.588	0.588	0.608	0.608	0.615	0.615	0.623	0.615	0.623	0.629	0.629	0.629

E_b = 30 ksi
h_{ac} = 6 inches

E _a c (ksi)	h _{EP} (inches)	Φ (deg. F)	M _f		M _u		M _f		M _u		M _f		M _u		M _f		M _u	
			d0	(mils)	d0	(mils)	d0	(mils)	d0	(mils)	d0	(mils)	d0	(mils)	d0	(mils)	d0	(mils)
2293.46	6.942	30	10.0162	7.6215	6.2751	5.3949	4.7669	4.2924	1.355	1.351	1.348	1.346	1.343	1.341	1.341	1.341	1.341	1.341
204.71	6.974	35	10.3032	7.8386	6.4529	5.5470	4.9007	4.4124	1.317	1.314	1.311	1.309	1.307	1.305	1.305	1.305	1.305	1.305
1850.36	7.012	40	10.6388	8.0922	6.6605	5.7245	5.0568	4.5523	1.275	1.273	1.270	1.268	1.266	1.265	1.265	1.265	1.265	1.265
1627.24	7.057	45	11.0258	8.3845	6.8996	5.9289	5.2364	4.7131	1.231	1.228	1.226	1.223	1.222	1.221	1.221	1.221	1.221	1.221
1411.23	7.108	50	11.4673	8.7178	7.1720	6.1616	5.4407	4.8961	1.183	1.181	1.180	1.178	1.177	1.176	1.176	1.176	1.176	1.176
1207.09	7.167	55	11.9670	9.0946	7.4798	6.4242	5.6712	5.1023	1.134	1.132	1.131	1.130	1.129	1.128	1.128	1.128	1.128	1.128
1018.40	7.235	60	12.5288	9.5178	7.8251	6.7187	5.9294	5.3332	1.083	1.082	1.081	1.080	1.079	1.079	1.079	1.079	1.079	1.079
847.58	7.313	65	13.1570	9.9903	8.2102	7.0468	6.2168	5.5899	1.031	1.031	1.031	1.030	1.030	1.030	1.030	1.030	1.030	1.030
754.20	7.365	68	13.5677	10.2989	8.4615	7.2606	6.4040	5.7570	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
695.91	7.403	70	13.8559	10.5153	8.6376	7.4103	6.5350	5.8739	0.979	0.979	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980
563.73	7.505	75	14.6302	11.0958	9.1094	7.8112	6.8854	6.1861	0.927	0.928	0.929	0.930	0.931	0.931	0.931	0.931	0.931	0.931
450.57	7.621	80	15.4842	11.7349	9.6278	8.2510	7.2692	6.5278	0.876	0.878	0.879	0.880	0.881	0.882	0.882	0.882	0.882	0.882
355.36	7.755	85	16.4223	12.4351	10.1947	8.7310	7.6874	6.8994	0.826	0.826	0.830	0.832	0.833	0.834	0.834	0.834	0.834	0.834
276.56	7.908	90	17.4480	13.1987	10.8114	9.2521	8.1405	7.3013	0.778	0.780	0.783	0.785	0.787	0.788	0.788	0.788	0.788	0.788
212.41	8.083	95	18.5643	14.0270	11.4786	9.8144	8.6283	7.7332	0.731	0.734	0.737	0.740	0.742	0.744	0.744	0.744	0.744	0.744
161.00	8.285	100	19.7730	14.9206	12.1961	10.4174	9.1502	8.1941	0.686	0.690	0.694	0.697	0.700	0.703	0.703	0.703	0.703	0.703
120.45	8.517	105	21.0739	15.8784	12.9623	11.0594	9.7042	8.6821	0.644	0.649	0.653	0.657	0.660	0.663	0.663	0.663	0.663	0.663
88.94	8.784	110	22.4652	16.8979	13.7747	11.7376	10.2876	9.1946	0.604	0.609	0.614	0.619	0.623	0.626	0.626	0.626	0.626	0.626
64.82	9.094	115	23.9427	17.9747	14.6289	12.4479	10.8965	9.7277	0.567	0.573	0.578	0.583	0.588	0.592	0.592	0.592	0.592	0.592
46.63	9.453	120	25.4991	19.1024	15.5190	13.1850	11.5259	10.2770	0.532	0.539	0.545	0.551	0.556	0.560	0.560	0.560	0.560	0.560

E _b = 30 ksi	8 inches
hac =	

E _a c (ksi)	hEp (inches)	t _p (deg F)	M _f														
			15 ksi	15 ksi	15 ksi	20 ksi	20 ksi	20 ksi	25 ksi	25 ksi	25 ksi	30 ksi	30 ksi	30 ksi	35 ksi	35 ksi	35 ksi
2293.46	8.942	30	7.8055	5.9462	4.9007	4.2172	3.7295	3.3610	1.376	1.374	1.372	1.370	1.369	1.367			
2074.71	8.974	35	6.0391	6.1235	5.0464	4.3422	3.8397	3.4601	1.336	1.334	1.333	1.331	1.329	1.328			
1850.36	9.012	40	6.3130	6.3313	5.2171	4.4886	3.9688	3.5761	1.290	1.289	1.288	1.286	1.285	1.285			
1627.24	9.057	45	8.6300	6.5717	5.4144	4.6578	4.1179	3.7100	1.245	1.243	1.242	1.241	1.240	1.239			
1411.23	9.108	50	8.9931	6.8469	5.6402	4.8513	4.2884	3.8631	1.194	1.193	1.192	1.191	1.190	1.190			
1207.09	9.167	55	9.4058	7.1596	5.8966	5.0710	4.4819	4.0368	1.142	1.141	1.141	1.140	1.139	1.138			
1018.40	9.235	60	9.8723	7.5127	6.1861	5.3188	4.7000	4.2324	1.088	1.088	1.087	1.087	1.086	1.086			
847.58	9.313	65	10.3970	7.9096	6.5111	5.5988	4.9445	4.4517	1.033	1.033	1.033	1.033	1.032	1.032			
754.20	9.365	68	10.7419	8.1702	6.7244	5.7792	5.1049	4.5954	1.000	1.000	1.000	1.000	1.000	1.000			
695.91	9.403	70	10.9847	8.3536	6.8744	5.9074	5.2176	4.6963	0.978	0.978	0.978	0.978	0.978	0.978			
563.73	9.505	75	11.6407	8.8487	7.2791	6.2530	5.5211	4.9681	0.923	0.923	0.924	0.924	0.925	0.925			
450.57	9.621	80	12.3704	9.3987	7.7281	6.6361	5.8571	5.2686	0.868	0.868	0.870	0.871	0.872	0.872			
355.36	9.755	85	13.1795	10.0075	8.2244	7.0590	6.2276	5.5997	0.815	0.816	0.818	0.819	0.820	0.821			
276.56	9.908	90	14.0738	10.6791	8.7710	7.5239	6.6345	5.9627	0.763	0.765	0.767	0.768	0.769	0.771			
212.41	10.083	95	15.0586	11.4170	9.3704	8.0329	7.0792	6.3589	0.713	0.713	0.716	0.718	0.721	0.723			
161.00	10.285	100	16.1389	12.2243	10.0246	8.5874	7.5626	6.7888	0.666	0.668	0.671	0.673	0.675	0.677			
120.45	10.517	105	17.3187	13.1033	10.7350	9.1880	8.0852	7.2527	0.620	0.624	0.626	0.629	0.631	0.634			
88.94	10.784	110	18.6007	14.0550	11.5019	9.8346	8.6464	7.7496	0.577	0.581	0.585	0.588	0.590	0.593			
64.82	11.094	115	19.9859	15.0791	12.3242	10.5257	9.2445	8.2780	0.537	0.542	0.546	0.549	0.552	0.555			
46.63	11.453	120	21.4727	16.1732	13.1992	11.2586	9.8788	8.8349	0.500	0.505	0.509	0.513	0.517	0.520			

Eb = 30 ksi
hac = 10 inches

Eac (ksi)	hEp (inches)	tp (deg. F)	W												M											
			13	14	15	16	17	18	19	20	21	22	23	13	14	15	16	17	18	19	20	21	22	23	24	
2393.46	10.942	30	d0 (mils)	4.8716	4.0172	3.4586	3.0600	2.7588	d0 (mils)	1.390	1.386	1.385	1.384	1.345	1.346	1.346	1.345	1.345	1.345	1.345	1.345	1.345	1.345	1.345	1.345	
2074.71	10.974	35	6.5874	5.0208	4.1400	3.5641	3.1531	2.8426	1.348	1.347	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	1.346	
1850.36	11.012	40	6.8181	5.1962	4.2842	3.6880	3.2625	2.9411	1.303	1.302	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	1.301	
1627.24	11.057	45	7.0854	5.3993	4.4513	3.8315	3.3892	3.0550	1.254	1.253	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	1.252	
1411.23	11.108	50	7.3925	5.6326	4.6311	3.9961	3.5345	3.1857	1.202	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	1.201	
1207.09	11.167	55	7.7425	5.8984	4.8615	4.1836	3.6999	3.3344	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	1.147	
1018.40	11.235	60	8.1394	6.1996	5.1090	4.3959	3.8872	3.5027	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	1.091	
847.58	11.313	65	8.5874	6.5395	5.3880	4.6352	4.0981	3.6922	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	1.034	
754.20	11.365	68	8.8829	6.7635	5.5718	4.7928	4.2369	3.8169	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
695.91	11.403	70	9.0914	6.9215	5.7015	4.9039	3.9048	3.9048	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	
563.73	11.505	75	9.6565	7.3495	6.0524	5.2044	4.5994	4.1423	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	0.920	
450.57	11.621	80	10.2886	7.8277	6.4442	5.5397	4.8944	4.4069	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	0.863	
355.36	11.755	85	10.9937	8.3606	6.8803	5.9126	5.2222	4.7006	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	0.808	
276.56	11.908	90	11.7784	8.9528	7.3643	6.3260	5.5852	5.0256	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	0.754	
212.41	12.083	95	12.6491	9.6089	7.8998	6.7827	5.9858	5.3838	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	0.702	
161.00	12.285	100	13.6126	10.3334	8.4901	7.2854	6.4261	5.7770	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	0.653	
120.45	12.517	105	14.6750	11.1304	9.1383	7.8364	6.9079	6.2066	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	0.605	
88.94	12.784	110	15.8420	12.0036	9.8466	8.4372	7.4322	6.6733	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	0.561	
64.82	13.094	115	17.1180	12.9552	10.6165	9.0887	7.9995	7.1773	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	0.519	
46.63	13.453	120	18.5057	13.9864	11.4481	9.7904	8.6090	7.7174	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	0.480	

E_b = 30 ksi
hac = 12 inches

E _a (ksi)	h _{Ep} (inches)	tp (deg. F.)	M ₁₅			M ₂₀			M ₂₅			M ₃₀			M ₃₅			M ₄₀			M ₄₅		
			M ₁₅ ksi	M ₁₅ kips	d ₀ (mils)	M ₂₀ ksi	M ₂₀ kips	d ₀ (mils)	M ₂₅ ksi	M ₂₅ kips	d ₀ (mils)	M ₃₀ ksi	M ₃₀ kips	d ₀ (mils)	M ₃₅ ksi	M ₃₅ kips	d ₀ (mils)	M ₄₀ ksi	M ₄₀ kips	d ₀ (mils)	M ₄₅ ksi	M ₄₅ kips	d ₀ (mils)
2293.46	12.942	30	5.4095	4.1248	3.4024	2.9301	2.5930	2.3384	1.939	1.398	1.397	1.396	1.396	1.395	1.395	1.395	1.395	1.353	1.353	1.353	1.353	1.353	1.353
2074.71	12.974	35	5.5786	4.2534	3.5083	3.0212	2.6736	2.4109	1.357	1.356	1.355	1.354	1.354	1.353	1.353	1.353	1.353	1.308	1.308	1.308	1.308	1.308	1.308
1850.36	13.012	40	5.7774	4.4047	3.6329	3.1283	2.7683	2.4962	1.310	1.309	1.308	1.308	1.308	1.308	1.308	1.308	1.308	1.258	1.258	1.258	1.258	1.258	1.258
1627.24	13.057	45	6.0082	4.5804	3.7775	3.2526	2.8781	2.5951	1.260	1.259	1.259	1.258	1.258	1.258	1.258	1.258	1.258	1.257	1.257	1.257	1.257	1.257	1.257
1411.23	13.108	50	6.2738	4.7824	3.9438	3.3955	3.0043	2.7087	1.207	1.206	1.206	1.205	1.205	1.205	1.205	1.205	1.205	1.204	1.204	1.204	1.204	1.204	1.204
1207.09	13.167	55	6.5771	5.0130	4.1336	3.5586	3.1483	2.8383	1.151	1.150	1.150	1.150	1.150	1.150	1.150	1.150	1.150	1.149	1.149	1.149	1.149	1.149	1.149
1018.40	13.235	60	6.9217	5.2750	4.3491	3.7437	3.3118	2.9854	1.094	1.094	1.093	1.093	1.093	1.093	1.093	1.093	1.093	1.092	1.092	1.092	1.092	1.092	1.092
847.58	13.313	65	7.3117	5.5713	4.5928	3.9530	3.4965	3.1515	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035	1.035
754.20	13.365	68	7.5694	5.7671	4.7537	4.0911	3.6184	3.2612	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
695.91	13.403	70	7.7516	5.9054	4.8674	4.1887	3.7044	3.3385	0.976	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977	0.977
563.73	13.505	75	8.2465	6.2811	5.1759	4.4534	3.9379	3.5484	0.918	0.918	0.918	0.918	0.918	0.918	0.918	0.918	0.918	0.919	0.919	0.919	0.919	0.919	0.919
450.57	13.621	80	8.8021	6.7024	5.5219	4.7500	4.1993	3.7832	0.860	0.860	0.860	0.860	0.860	0.860	0.860	0.860	0.860	0.862	0.862	0.862	0.862	0.862	0.862
355.36	13.755	85	9.4245	7.1741	5.9087	5.0815	4.4913	4.0454	0.803	0.803	0.804	0.804	0.804	0.805	0.805	0.805	0.806	0.806	0.806	0.806	0.806	0.806	0.806
276.56	13.908	90	10.1204	7.7098	6.3405	5.4511	4.9166	4.3372	0.748	0.749	0.749	0.750	0.751	0.751	0.751	0.751	0.752	0.752	0.752	0.752	0.752	0.752	0.752
212.41	14.083	95	10.8968	8.2878	6.8210	5.8621	5.1780	4.6611	0.695	0.696	0.696	0.697	0.697	0.697	0.697	0.697	0.698	0.698	0.698	0.698	0.698	0.698	0.698
161.00	14.285	100	11.7611	8.9402	7.3544	6.3178	5.5782	5.0195	0.644	0.645	0.645	0.646	0.646	0.646	0.646	0.646	0.649	0.649	0.649	0.649	0.649	0.649	0.649
120.45	14.517	105	12.7206	9.6633	7.9447	6.8213	6.0199	5.4146	0.595	0.597	0.597	0.598	0.598	0.600	0.600	0.600	0.601	0.601	0.601	0.601	0.601	0.601	0.601
88.94	14.784	110	13.7827	10.4620	8.5954	7.3755	6.5054	5.8481	0.549	0.551	0.551	0.555	0.555	0.555	0.555	0.555	0.556	0.556	0.556	0.556	0.556	0.556	0.556
64.82	15.094	115	14.9541	11.3407	9.3098	7.9827	7.0363	6.3215	0.506	0.509	0.509	0.511	0.511	0.512	0.512	0.512	0.514	0.514	0.514	0.514	0.514	0.514	0.514
46.63	15.453	120	16.2404	12.3027	10.0901	8.6444	7.6137	6.8354	0.466	0.469	0.469	0.471	0.471	0.471	0.471	0.471	0.475	0.475	0.475	0.475	0.475	0.475	0.475

Eac (ksi)	hEp (inches)	tp (deg. F.)	W			M			N			P			Q			R		
			15	20	25	30	35	40	45	50	55	60	65	70	75	80	85	90	95	100
2293.46	3.187	30	21.0821	15.8835	d0 (mils)	TAF	TAF	TAF	TAF	TAF	TAF	TAF	TAF	TAF						
2074.71	3.228	35	21.4877	16.1811	13.2031	11.2600	9.8764	8.8331	8.0040	7.0710	6.0710	5.0040	4.166	3.185	2.191	1.207	1.201	1.196	1.191	1.187
1850.36	3.276	40	21.9520	16.5213	13.4741	11.4862	10.0710	9.0040	8.0040	7.0710	6.0710	5.0040	4.160	3.179	2.190	1.160	1.174	1.179	1.170	1.167
1627.24	3.331	45	22.4745	16.9033	13.7780	11.7395	10.2886	9.1948	8.1948	7.1395	6.1395	5.1395	4.134	3.134	2.134	1.134	1.130	1.135	1.155	1.144
1411.23	3.396	50	23.0543	17.3264	14.1140	12.0191	10.5284	9.4049	8.4049	7.4049	6.4049	5.4049	4.110	3.110	2.110	1.106	1.103	1.100	1.098	1.096
1207.09	3.471	55	23.6905	17.7895	14.4809	12.3240	10.7895	9.6333	8.6333	7.6333	6.6333	5.6333	4.080	3.078	2.078	1.075	1.073	1.071	1.070	1.070
1018.40	3.556	60	24.3813	18.2911	14.8775	12.6528	11.0706	9.8789	8.8789	7.8789	6.8789	5.8789	4.050	3.048	2.048	1.046	1.045	1.044	1.043	1.043
847.58	3.655	65	25.1248	18.8293	15.3019	13.0040	11.3702	10.1401	9.1401	8.1401	7.1401	6.1401	4.019	3.018	2.018	1.017	1.017	1.016	1.016	1.016
754.20	3.720	68	25.5951	19.1689	15.5692	13.2247	11.5582	10.3038	9.3038	8.3038	7.3038	6.3038	4.000	3.000	2.000	1.000	1.000	1.000	1.000	1.000
695.91	3.767	70	25.9179	19.4016	15.7520	13.3755	11.6866	10.4155	9.4155	8.4155	7.4155	6.4155	4.088	3.088	2.088	0.988	0.988	0.989	0.989	0.989
563.73	3.896	75	26.7573	20.0050	16.2253	13.7651	12.0176	10.7030	9.7030	8.7030	7.7030	6.7030	4.057	3.058	2.058	0.960	0.961	0.962	0.963	0.963
450.57	4.043	80	27.6384	20.6361	16.7186	14.1702	12.3669	11.0005	9.0005	8.0005	7.0005	6.0005	4.026	3.029	2.029	0.931	0.933	0.935	0.937	0.937
355.36	4.211	85	28.5560	21.2905	17.2284	14.5876	12.7137	11.3056	9.904	8.904	7.904	6.904	4.096	3.096	2.096	0.907	0.909	0.911	0.911	0.911
276.56	4.404	90	29.5040	21.9637	17.7509	15.0140	13.0733	11.6157	10.873	9.873	8.873	7.873	4.068	3.068	2.068	0.877	0.881	0.884	0.887	0.887
212.41	4.625	95	30.4755	22.6502	18.2818	15.4459	13.4363	11.9282	10.846	9.846	8.846	7.846	4.046	3.046	2.046	0.852	0.855	0.856	0.864	0.864
161.00	4.879	100	31.4628	23.3445	18.8166	15.8795	13.7998	12.2401	10.814	9.814	8.814	7.814	4.021	3.027	2.027	0.833	0.838	0.842	0.842	0.842
120.45	5.171	105	32.4578	24.0407	19.3506	16.3110	14.1605	12.5489	10.789	9.789	8.789	7.789	4.014	3.014	2.014	0.805	0.811	0.816	0.821	0.821
88.94	5.508	110	33.4519	24.7327	19.8792	16.7367	14.5154	12.8519	10.765	9.765	8.765	7.765	4.007	3.007	2.007	0.783	0.790	0.796	0.802	0.802
64.82	5.898	115	34.4368	25.4147	20.3981	17.1532	14.8615	13.1468	10.743	9.743	8.743	7.743	4.000	3.000	2.000	0.771	0.771	0.778	0.784	0.784
46.63	6.351	120	35.4041	26.0812	20.9032	17.5573	15.1965	13.4316	10.723	9.723	8.723	7.723	4.000	3.000	2.000	0.745	0.753	0.761	0.767	0.767

E _{ac} (ksi)	h/E _P (inches)	Ip (deg. F)	d0 (mils)	TAF	TAF	TAF	TAF	TAF	TAF						
2293.46	5.187	30	13.3056	10.1020	8.3011	7.1241	6.2845	5.6503	5.298	1.286	1.286	1.283	1.280	1.283	1.280
2074.71	5.228	35	13.6396	10.3528	8.5053	7.2978	6.4365	5.7860	1.266	1.262	1.259	1.256	1.253	1.250	1.250
1850.36	5.276	40	14.0270	10.6436	8.7418	7.4989	6.6124	5.9428	1.231	1.228	1.225	1.222	1.219	1.217	1.217
1622.24	5.331	45	14.4697	10.9755	9.0115	7.7281	6.8127	6.1213	1.194	1.191	1.188	1.186	1.184	1.182	1.182
1411.23	5.396	50	14.9696	11.3498	9.3154	7.9860	7.0380	6.3219	1.154	1.151	1.149	1.147	1.146	1.144	1.144
1207.09	5.471	55	15.5286	11.7679	9.6545	8.2735	7.2887	6.5451	1.112	1.111	1.109	1.108	1.106	1.105	1.105
1018.40	5.556	60	16.1489	12.2310	10.0295	8.5911	7.5655	6.7910	1.070	1.068	1.067	1.066	1.065	1.065	1.065
847.58	5.655	65	16.8322	12.7403	10.4412	8.9393	7.8685	7.0601	1.026	1.026	1.025	1.025	1.025	1.024	1.024
754.20	5.720	68	17.2733	13.0685	10.7062	9.1631	8.0631	7.2326	1.000	1.000	1.000	1.000	1.000	1.000	1.000
695.91	5.767	70	17.5801	13.2966	10.8902	9.3183	8.1979	7.3521	0.983	0.983	0.983	0.983	0.984	0.984	0.984
563.73	5.896	75	18.3937	13.9002	11.3764	9.7281	8.5534	7.6668	0.939	0.940	0.941	0.942	0.943	0.943	0.943
450.57	6.043	80	19.2734	14.5513	11.8995	10.1681	8.9344	8.0034	0.896	0.898	0.900	0.901	0.904	0.904	0.904
355.36	6.211	85	20.2188	15.2488	12.4586	10.6372	9.3398	8.3610	0.854	0.857	0.859	0.861	0.863	0.865	0.865
276.56	6.404	90	21.2285	15.9913	13.0519	11.1339	9.7680	8.7380	0.814	0.817	0.820	0.823	0.825	0.828	0.828
212.41	6.625	95	22.2998	16.7760	13.6771	11.6557	10.2168	9.1321	0.775	0.779	0.783	0.786	0.789	0.792	0.792
161.00	6.879	100	23.4284	17.5995	14.3308	12.1997	10.6834	9.5408	0.737	0.743	0.747	0.751	0.755	0.758	0.758
120.45	7.171	105	24.6087	18.4567	15.0088	12.6087	11.1643	9.9610	0.702	0.708	0.713	0.718	0.722	0.726	0.726
88.94	7.508	110	25.8333	19.3417	15.7059	13.3382	11.6555	10.3889	0.669	0.676	0.682	0.687	0.692	0.696	0.696
64.82	7.898	115	27.0933	20.2476	16.4164	13.9233	12.1526	10.8208	0.638	0.645	0.652	0.658	0.663	0.668	0.668
46.63	8.351	120	28.3783	21.1665	17.1338	14.5118	12.6511	11.2526	0.609	0.617	0.625	0.631	0.637	0.643	0.643

E_b = 60 ksi
hac = 6 inches

E_a (ksi)	hEP (inches)	t_p (deg. F)	M₁₃			M₁₅			M₁₇			M₁₉			M₂₁			M₂₃			M₂₅			M₂₇			M₂₉			M₃₁			M₃₃			M₃₅			M₃₇			M₃₉			M₄₁			M₄₃			M₄₅			M₄₇			M₄₉			M₅₁			M₅₃			M₅₅			M₅₇			M₅₉			M₆₁			M₆₃			M₆₅			M₆₇			M₆₉			M₇₁			M₇₃			M₇₅			M₇₇			M₇₉			M₈₁			M₈₃			M₈₅			M₈₇			M₈₉			M₉₁			M₉₃			M₉₅			M₉₇			M₉₉			M₁₀₁			M₁₀₃			M₁₀₅			M₁₀₇			M₁₀₉			M₁₁₁			M₁₁₃			M₁₁₅			M₁₁₇			M₁₁₉			M₁₂₁			M₁₂₃			M₁₂₅			M₁₂₇			M₁₂₉			M₁₃₁			M₁₃₃			M₁₃₅			M₁₃₇			M₁₃₉			M₁₄₁			M₁₄₃			M₁₄₅			M₁₄₇			M₁₄₉			M₁₅₁			M₁₅₃			M₁₅₅			M₁₅₇			M₁₅₉			M₁₆₁			M₁₆₃			M₁₆₅			M₁₆₇			M₁₆₉			M₁₇₁			M₁₇₃			M₁₇₅			M₁₇₇			M₁₇₉			M₁₈₁			M₁₈₃			M₁₈₅			M₁₈₇			M₁₈₉			M₁₉₁			M₁₉₃			M₁₉₅			M₁₉₇			M₁₉₉			M₂₀₁			M₂₀₃			M₂₀₅			M₂₀₇			M₂₀₉			M₂₁₁			M₂₁₃			M₂₁₅			M₂₁₇			M₂₁₉			M₂₂₁			M₂₂₃			M₂₂₅			M₂₂₇			M₂₂₉			M₂₃₁			M₂₃₃			M₂₃₅			M₂₃₇			M₂₃₉			M₂₄₁			M₂₄₃			M₂₄₅			M₂₄₇			M₂₄₉			M₂₅₁			M₂₅₃			M₂₅₅			M₂₅₇			M₂₅₉			M₂₆₁			M₂₆₃			M₂₆₅			M₂₆₇			M₂₆₉			M₂₇₁			M₂₇₃			M₂₇₅			M₂₇₇			M₂₇₉			M₂₈₁			M₂₈₃			M₂₈₅			M₂₈₇			M₂₈₉			M₂₉₁			M₂₉₃			M₂₉₅			M₂₉₇			M₂₉₉			M₃₀₁			M₃₀₃			M₃₀₅			M₃₀₇			M₃₀₉			M₃₁₁			M₃₁₃			M₃₁₅			M₃₁₇			M₃₁₉			M₃₂₁			M₃₂₃			M₃₂₅			M₃₂₇			M₃₂₉			M₃₃₁			M₃₃₃			M₃₃₅			M₃₃₇			M₃₃₉			M₃₄₁			M₃₄₃			M₃₄₅			M₃₄₇			M₃₄₉			M₃₅₁			M₃₅₃			M₃₅₅			M₃₅₇			M₃₅₉			M₃₆₁			M₃₆₃			M₃₆₅			M₃₆₇			M₃₆₉			M₃₇₁			M₃₇₃			M₃₇₅			M₃₇₇			M₃₇₉			M₃₈₁			M₃₈₃			M₃₈₅			M₃₈₇			M₃₈₉			M₃₉₁			M₃₉₃			M₃₉₅			M₃₉₇			M₃₉₉			M₄₀₁			M₄₀₃			M₄₀₅			M₄₀₇			M₄₀₉			M₄₁₁			M₄₁₃			M₄₁₅			M₄₁₇			M₄₁₉			M₄₂₁			M₄₂₃			M₄₂₅			M₄₂₇			M₄₂₉			M₄₃₁			M₄₃₃			M₄₃₅			M₄₃₇			M₄₃₉			M₄₄₁			M₄₄₃			M₄₄₅			M₄₄₇			M₄₄₉			M₄₅₁			M₄₅₃			M₄₅₅			M₄₅₇			M₄₅₉			M₄₆₁			M₄₆₃			M₄₆₅			M₄₆₇			M₄₆₉			M₄₇₁			M₄₇₃			M₄₇₅			M₄₇₇			M₄₇₉			M₄₈₁			M₄₈₃			M₄₈₅			M₄₈₇			M₄₈₉			M₄₉₁			M₄₉₃			M₄₉₅			M₄₉₇			M₄₉₉			M₅₀₁			M₅₀₃			M₅₀₅			M₅₀₇			M₅₀₉			M₅₁₁			M₅₁₃			M₅₁₅			M₅₁₇			M₅₁₉			M₅₂₁			M₅₂₃			M₅₂₅			M₅₂₇			M₅₂₉			M₅₃₁			M₅₃₃			M₅₃₅			M₅₃₇			M₅₃₉			M₅₄₁			M₅₄₃			M₅₄₅			M₅₄₇			M₅₄₉			M₅₅₁			M₅₅₃			M₅₅₅			M₅₅₇			M₅₅₉			M₅₆₁			M₅₆₃			M₅₆₅			M₅₆₇			M₅₆₉			M₅₇₁			M₅₇₃			M₅₇₅			M₅₇₇			M₅₇₉			M₅₈₁			M₅₈₃			M₅₈₅			M₅₈₇			M₅₈₉			M₅₉₁			M₅₉₃			M₅₉₅			M₅₉₇			M₅₉₉			M₆₀₁			M₆₀₃			M₆₀₅			M₆₀₇			M₆₀₉			M₆₁₁			M₆₁₃			M₆₁₅			M₆₁₇			M₆₁₉			M₆₂₁			M₆₂₃			M₆₂₅			M₆₂₇			M₆₂₉			M₆₃₁			M₆₃₃			M₆₃₅			M₆₃₇			M₆₃₉			M₆₄₁			M₆₄₃			M₆₄₅			M₆₄₇			M₆₄₉			M₆₅₁			M₆₅₃			M₆₅₅			M₆₅₇			M₆₅₉			M₆₆₁			M₆₆₃			M₆₆₅			M₆₆₇			M₆₆₉			M₆₇₁			M₆₇₃			M₆₇₅			M₆₇₇			M₆₇₉			M₆₈₁			M₆₈₃			M₆₈₅			M₆₈₇			M₆₈₉			M₆₉₁			M₆₉₃			M₆₉₅			M₆₉₇			M₆₉₉			M₇₀₁			M₇₀₃			M₇₀₅			M₇₀₇			M₇₀₉			M₇₁₁			M₇₁₃			M₇₁₅			M₇₁₇			M₇₁₉			M₇₂₁			M₇₂₃			M₇₂₅			M₇₂₇			M₇₂₉			M₇₃₁			M₇₃₃			M₇₃₅			M₇₃₇			M₇₃₉			M₇₄₁			M₇₄₃			M₇₄₅			M₇₄₇			M₇₄₉			M₇₅₁			M₇₅₃			M₇₅₅			M₇₅₇			M₇₅₉			M₇₆₁			M₇₆₃			M₇₆₅			M₇₆₇			M₇₆₉			M₇₇₁			M₇₇₃			M₇₇₅			M₇₇₇			M₇₇₉			M₇₈₁			M₇₈₃			M₇₈₅			M₇₈₇			M₇₈₉			M₇₉₁			M₇₉₃			M₇₉₅			M₇₉₇			M₇₉₉			M₈₀₁			M₈₀₃			M₈₀₅			M₈₀₇			M₈₀₉			M₈₁₁			M₈₁₃			M₈₁₅			M₈₁₇			M₈₁₉			M₈₂₁			M₈₂₃			M₈₂₅			M₈₂₇			M₈₂₉			M₈₃₁			M₈₃₃			M₈₃₅			M₈₃₇			M₈₃₉			M₈₄₁			M₈₄₃			M₈₄₅			M₈₄₇			M₈₄₉			M₈₅₁			M₈₅₃			M₈₅₅			M₈₅₇			M₈₅₉			M₈₆₁			M₈₆₃			M₈₆₅			M₈₆₇			M₈₆₉			M₈₇₁			M₈₇₃			M₈₇₅			M₈₇₇			M₈₇₉			M₈₈₁			M₈₈₃			M₈₈₅			M₈₈₇			M₈₈₉			M₈₉₁			M₈₉₃			M₈₉₅			M₈₉₇			M₈₉₉			M₉₀₁			M₉₀₃			M₉₀₅			M₉₀₇			M₉₀₉			M₉₁₁			M₉₁₃			M₉₁₅			M₉₁₇			M₉₁₉			M₉₂₁			M₉₂₃			M₉₂₅			M₉₂₇			M₉₂₉			M₉₃₁			M₉₃₃			M₉₃₅			M₉₃₇			M₉₃₉			M₉₄₁			M₉₄₃			M₉₄₅			M₉₄₇			M₉₄₉			M₉₅₁			M₉₅₃			M₉₅₅			M₉₅₇			M₉₅₉			M₉₆₁			M₉₆₃			M₉₆₅			M₉₆₇			M₉₆₉			M₉₇₁			M₉₇₃			M₉₇₅			M₉₇₇			M₉₇₉			M₉₈₁			M₉₈₃			M₉₈₅			M₉₈₇			M₉₈₉			M₉₉₁			M₉₉₃			M₉₉₅			M₉₉₇			M₉₉₉			M₁₀₀₁			M₁₀₀₃			M₁₀₀₅			M₁₀₀₇			M₁₀₀₉			M₁₀₁₁			M₁₀₁₃			M₁₀₁₅			M₁₀₁₇			M₁₀₁₉			M₁₀₂₁			M₁₀₂₃			M₁₀₂₅			M₁₀₂₇			M₁₀₂₉			M₁₀₃₁			M₁₀₃₃			M₁₀₃₅			M₁₀₃₇			M₁₀₃₉			M₁₀₄₁			M₁₀₄₃			M₁₀₄₅			M₁₀₄₇			M₁₀₄₉			M₁₀₅₁			M₁₀₅₃			M₁₀₅₅			M₁₀₅₇			M₁₀₅₉			M₁₀₆₁			M₁₀₆₃			M₁₀₆₅			M₁₀₆₇			M₁₀₆₉			M₁₀₇₁			M₁₀₇₃			M₁₀₇₅			M₁₀₇₇			M₁₀₇₉			M₁₀₈₁			M₁₀₈₃			M₁₀₈₅			M₁₀₈₇			M₁₀₈₉			M₁₀₉₁			M₁₀₉₃			M₁₀₉₅			M₁₀₉₇			M₁₀₉₉			M₁₁₀₁			M₁₁₀₃			M₁₁₀₅			M₁₁₀₇			M₁₁₀₉			M₁₁₁₁			M₁₁₁₃			M₁₁₁₅			M₁₁₁₇			M₁₁₁₉			M₁₁₂₁			M₁₁₂₃			M₁₁₂₅			M₁₁₂₇			M₁₁₂₉			M₁₁₃₁			M₁₁₃₃			M₁₁₃₅			M₁₁₃₇			M₁₁₃₉			M₁₁₄₁			M₁₁₄₃			M₁₁₄₅			M₁₁₄₇			M₁₁₄₉			M₁₁₅₁			M₁₁₅₃			M₁₁₅₅			M₁₁₅₇			M₁₁₅₉			M₁₁₆₁			M₁₁₆₃			M₁₁₆₅			M₁₁₆₇			M₁₁₆₉			M₁₁₇₁			M₁₁₇₃			M₁₁₇₅			M₁₁₇₇			M₁₁₇₉			M₁₁₈₁			M₁₁₈₃			M₁₁₈₅			M₁₁₈₇			M₁₁₈₉			M₁₁₉₁			M₁₁₉₃			M₁₁₉₅			M₁₁₉₇			M₁₁₉₉			M₁₂₀₁			M₁₂₀₃			M₁₂₀₅			M₁₂₀₇			M₁₂₀₉			M₁₂₁₁			M₁₂₁₃			M₁₂₁₅			M₁₂₁₇			M₁₂₁₉			M₁₂₂₁			M₁₂₂₃			M₁₂₂₅			M₁₂₂₇			M₁₂₂₉			M₁₂₃₁			M₁₂₃₃			M₁₂₃₅			M₁₂₃₇			M₁₂₃₉			M₁₂₄₁			M₁₂₄₃			M₁₂₄₅			M₁₂₄₇			M₁₂₄₉			M₁₂₅₁			M₁₂₅₃			M₁₂₅₅			M₁₂₅₇			M₁₂₅₉			M₁₂₆₁			M₁₂₆₃			M₁₂₆₅			M₁₂₆₇			M₁₂₆₉			M₁₂₇₁			M₁₂₇₃			M₁₂₇₅			M₁₂₇₇			M₁₂₇₉			M₁₂₈₁			M₁₂₈₃			M₁₂₈₅			M₁₂₈₇			M₁₂₈₉			M₁₂₉₁			M₁₂₉₃			M₁₂₉₅			M₁₂₉₇			M₁₂₉₉			M₁₃₀₁			M₁₃₀₃			M₁₃₀₅			M₁₃₀₇			M₁₃₀₉			M₁₃₁₁			M₁₃₁₃			M₁₃₁₅			M₁₃₁₇			M₁₃₁₉			M_{1321</sub}		

E_b = 60 ksi
hac = 10 inches

E _a (ksi)	h/E _b (inches)	t _p (deg. F)	M _s											
			15	16	17	18	19	20	21	22	23	24	25	26
2293.46	11.187	30	6.2522	4.7660	3.9303	3.3839	2.9940	2.6994	2.3778	1.337	1.376	1.375	1.374	1.373
2074.71	11.228	35	6.4400	4.9088	4.0478	3.4848	3.0832	2.7797	1.338	1.336	1.335	1.334	1.333	1.333
1850.36	11.276	40	6.6603	5.0762	4.1856	3.6032	3.1877	2.8738	1.294	1.292	1.291	1.290	1.289	1.289
1627.24	11.331	45	6.9153	5.2700	4.3450	3.7402	3.3086	2.9825	1.246	1.245	1.244	1.244	1.243	1.242
1411.23	11.396	50	7.2075	5.4921	4.5276	3.8970	3.4470	3.1070	1.195	1.195	1.194	1.194	1.193	1.192
1207.09	11.471	55	7.5400	5.7446	4.7352	4.0752	3.6043	3.2484	1.143	1.142	1.142	1.141	1.141	1.141
1018.40	11.556	60	7.9160	6.0301	4.9697	4.2765	3.7818	3.4081	1.089	1.088	1.088	1.088	1.087	1.087
847.58	11.655	65	8.3393	6.3513	5.2335	4.5028	3.9814	3.5874	1.033	1.033	1.033	1.033	1.033	1.033
754.20	11.720	68	8.6177	6.5625	5.4069	4.6514	4.1124	3.7051	1.000	1.000	1.000	1.000	1.000	1.000
695.91	11.767	70	8.8139	6.7112	5.5290	4.7561	4.2046	3.7879	0.978	0.978	0.978	0.978	0.978	0.978
563.73	11.896	75	9.3444	7.1132	5.8587	5.0386	4.4534	4.0113	0.922	0.923	0.923	0.923	0.923	0.924
450.57	12.043	80	9.9354	7.5607	6.2255	5.3526	4.7298	4.2593	0.867	0.868	0.869	0.869	0.869	0.870
355.36	12.211	85	10.5920	8.0572	6.6321	5.7005	5.0358	4.5336	0.814	0.814	0.815	0.816	0.817	0.817
276.56	12.404	90	11.3192	8.6065	7.0814	6.0845	5.3732	4.8359	0.761	0.763	0.764	0.764	0.765	0.766
212.41	12.625	95	12.1222	9.2121	7.5761	6.5068	5.7439	5.1676	0.711	0.712	0.714	0.715	0.716	0.717
161.00	12.879	100	13.0057	9.8773	8.1188	6.9694	6.1495	5.5301	0.663	0.664	0.666	0.667	0.669	0.670
120.45	13.171	105	13.9740	10.6050	8.7113	7.4737	6.5910	5.9243	0.617	0.619	0.621	0.622	0.624	0.625
88.94	13.508	110	15.0309	11.3973	9.3552	8.0208	7.0691	6.3504	0.573	0.576	0.578	0.580	0.582	0.583
64.82	13.898	115	16.1785	12.2554	10.0510	8.6107	7.5837	6.8083	0.533	0.535	0.538	0.540	0.542	0.544
46.63	14.351	120	17.4177	13.1792	10.7980	9.2426	8.1338	7.2967	0.495	0.498	0.501	0.503	0.506	0.508

$$\begin{array}{ll} E_b = & 60 \text{ ksi} \\ hac = & 12 \text{ inches} \end{array}$$

Eac (ksi)	hEp (inches)	tp (deg. F)	M1	M2	M3	M4	M5	M6	M7	M8	M9	M10	M11	M12	M13	M14	M15	M16	M17	M18	M19	M20	M21	M22	M23	M24	M25	M26	M27	M28	M29	M30	M31	M32	M33	M34	M35	M36	M37	M38	M39	M40	M41	M42	M43	M44	M45	M46
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E_b = 90 ksi
hac = 2 inches

E _a (ksi)	hEP (inches)	t _p (deg F)	M ₁₅														
			M ₁₅														
2293.46	3.359	30	20.0878	15.1520	d0 (mils)	12.3809	10.5720	9.2834	8.3112	7.45	6.65	5.85	5.05	4.25	3.45	2.65	1.85
2074.71	3.405	35	20.4573	15.4241	d0 (mils)	12.5987	10.7545	9.4409	8.4500	7.6077	6.8201	6.023	5.23	4.43	3.63	2.83	2.03
1850.36	3.460	40	20.8797	15.7348	d0 (mils)	12.8470	10.9623	9.6201	8.7838	8.0203	7.2312	6.432	5.633	4.834	4.035	3.236	2.437
1627.24	3.524	45	21.3542	16.0832	d0 (mils)	13.1250	11.1948	9.8203	8.9775	8.2048	7.4051	6.606	5.807	5.008	4.209	3.410	2.611
1411.23	3.598	50	21.8798	16.4684	d0 (mils)	13.4321	11.4511	10.0408	9.2806	8.5206	7.7203	6.9201	6.1200	5.3199	4.5197	3.7195	2.9193
1207.09	3.684	55	22.4554	16.8894	d0 (mils)	13.7669	11.7303	10.2806	9.1879	8.4206	7.6203	6.8201	6.0199	5.2197	4.4195	3.6193	2.8191
1018.40	3.782	60	23.0791	17.3445	d0 (mils)	14.1283	12.0311	10.5386	9.4139	8.6542	7.8134	7.0542	6.2541	5.4540	4.6539	3.8538	3.0537
847.58	3.894	65	23.7489	17.8320	d0 (mils)	14.5145	12.3519	11.8542	10.8134	9.9857	9.0847	8.1846	7.2845	6.3844	5.4843	4.5842	3.6841
754.20	3.969	68	24.1718	18.1392	d0 (mils)	14.7575	12.5534	11.9857	10.9857	9.9074	9.0074	8.088	7.188	6.288	5.388	4.488	3.588
695.91	4.023	70	24.4618	18.3495	d0 (mils)	14.9236	12.6910	11.1032	10.1032	9.0232	8.1232	7.2231	6.3231	5.4230	4.5230	3.6230	2.7230
563.73	4.170	75	25.2145	18.8942	d0 (mils)	15.3531	13.0463	11.4063	10.1716	9.9559	9.0559	8.1558	7.2558	6.3558	5.4558	4.5558	3.6558
450.57	4.338	80	26.0029	19.4628	d0 (mils)	15.8002	13.4152	11.7204	10.4449	9.930	9.030	8.1302	7.2302	6.3302	5.4302	4.5302	3.6302
355.36	4.531	85	26.8220	20.0515	d0 (mils)	16.2617	13.7952	12.0431	10.7252	9.901	9.001	8.1010	7.2010	6.3010	5.4010	4.5010	3.6010
276.56	4.751	90	27.6664	20.6562	d0 (mils)	16.7343	14.1832	12.3720	11.0101	8.874	8.074	7.1740	6.2740	5.3740	4.4740	3.5740	2.6740
212.41	5.004	95	28.5298	21.2720	d0 (mils)	17.2141	14.5760	12.7041	11.2973	8.847	8.047	7.1472	6.2472	5.3472	4.4472	3.5472	2.6472
161.00	5.295	100	29.4055	21.8941	d0 (mils)	17.6972	14.9704	13.0367	11.5843	8.828	8.028	7.128	6.228	5.328	4.428	3.528	2.628
120.45	5.630	105	30.2865	22.5173	d0 (mils)	18.1795	15.3630	13.3670	11.8687	8.798	8.006	7.106	6.206	5.306	4.406	3.506	2.606
88.94	6.016	110	31.1653	23.1363	d0 (mils)	18.6569	15.7506	13.6922	12.1482	8.776	8.076	7.176	6.276	5.376	4.476	3.576	2.676
64.82	6.462	115	32.0348	23.7462	d0 (mils)	19.1257	16.1301	14.0100	12.4206	8.755	8.055	7.155	6.255	5.355	4.455	3.555	2.655
46.63	6.980	120	32.8880	24.3422	d0 (mils)	19.5824	16.4988	14.3181	12.6843	8.735	8.035	7.135	6.235	5.335	4.435	3.535	2.635

E_b = 90 ksi
hac = 4 inches

E _{ac} (ksi)	hEP (inches)	tp (deg. F)	N			M			P			W			H			V			U			S		
			t _s	t _b	d ₀	t _s	t _b	d ₀	t _s	t _b	d ₀	t _s	t _b	d ₀	t _s	t _b	d ₀	t _s	t _b	d ₀	t _s	t _b	d ₀	t _s	t _b	
2293.46	5.359	30	12.8926	9.7914	8.0481	6.9087	6.0959	5.4819	1.287	1.283	1.279	1.276	1.273	1.271	1.273	1.271	1.273	1.271	1.273	1.271	1.273	1.271	1.273	1.271	1.273	
2074.71	5.405	35	13.2063	10.0273	8.2403	7.0723	6.2392	5.6098	1.256	1.253	1.249	1.247	1.244	1.242	1.244	1.242	1.244	1.242	1.244	1.242	1.244	1.242	1.244	1.242	1.244	
1850.36	5.460	40	13.5696	10.3002	8.4625	7.2614	6.4047	5.7576	1.223	1.219	1.217	1.217	1.214	1.212	1.214	1.212	1.214	1.212	1.214	1.212	1.214	1.212	1.214	1.212	1.214	
1627.24	5.524	45	13.9840	10.6113	8.7156	7.4766	6.5929	5.9254	1.186	1.184	1.181	1.179	1.177	1.176	1.177	1.176	1.177	1.177	1.176	1.177	1.176	1.177	1.176	1.177	1.176	
1411.23	5.598	50	14.4510	10.9615	9.0002	7.7184	6.8043	6.1138	1.148	1.146	1.144	1.142	1.141	1.139	1.141	1.139	1.141	1.141	1.141	1.142	1.141	1.141	1.142	1.141	1.142	
1207.09	5.684	55	14.9722	11.3518	9.3170	7.9874	7.0392	6.3230	1.108	1.108	1.106	1.105	1.104	1.103	1.104	1.103	1.104	1.103	1.103	1.104	1.103	1.104	1.103	1.104	1.103	
1018.40	5.782	60	15.5490	11.7832	9.6668	8.2840	7.2979	6.5532	1.067	1.066	1.065	1.064	1.064	1.063	1.064	1.063	1.064	1.063	1.064	1.064	1.063	1.064	1.063	1.064	1.063	
847.58	5.894	65	16.1829	12.2564	10.0500	8.6085	7.5806	6.8045	1.025	1.025	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	1.024	
754.20	5.969	68	16.5911	12.5607	10.2962	8.8167	7.7678	6.9654	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
695.91	6.023	70	16.8746	12.7719	10.4668	8.9609	7.8873	7.0767	0.983	0.983	0.983	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	0.984	
563.73	6.170	75	17.6249	13.3298	10.9170	9.3410	8.2176	7.3695	0.941	0.942	0.943	0.943	0.944	0.945	0.944	0.945	0.944	0.945	0.944	0.945	0.945	0.944	0.945	0.945	0.944	
450.57	6.338	80	18.4335	13.9298	11.4002	9.7482	8.5708	7.6822	0.900	0.902	0.903	0.903	0.904	0.906	0.904	0.906	0.905	0.906	0.906	0.906	0.906	0.906	0.906	0.906		
355.36	6.531	85	19.2996	14.5707	11.9151	10.1813	8.9458	8.0136	0.860	0.862	0.864	0.864	0.866	0.868	0.866	0.868	0.866	0.868	0.866	0.868	0.866	0.868	0.866	0.868		
276.56	6.751	90	20.2214	15.2508	12.4602	10.6387	9.3411	8.3622	0.820	0.824	0.826	0.826	0.828	0.830	0.826	0.828	0.826	0.828	0.826	0.828	0.826	0.828	0.826	0.828		
212.41	7.004	95	21.1958	15.9674	13.0330	11.1181	9.7555	8.7262	0.783	0.787	0.790	0.790	0.793	0.796	0.793	0.796	0.793	0.796	0.793	0.796	0.793	0.796	0.793	0.796		
161.00	7.295	100	22.2187	16.7170	13.6303	11.6168	10.1835	9.1030	0.747	0.751	0.755	0.755	0.759	0.762	0.759	0.762	0.759	0.762	0.759	0.762	0.759	0.762	0.759	0.762		
120.45	7.630	105	23.2846	17.4950	14.2483	12.1313	10.6250	9.4898	0.713	0.718	0.723	0.723	0.727	0.731	0.727	0.731	0.727	0.731	0.727	0.731	0.727	0.731	0.727	0.731		
88.94	8.016	110	24.3865	18.2961	14.8823	12.6576	11.0753	9.8836	0.680	0.687	0.692	0.692	0.697	0.701	0.697	0.701	0.697	0.701	0.697	0.698	0.697	0.698	0.697	0.698		
64.82	8.462	115	25.5163	19.1138	15.5273	13.1912	11.5307	10.2807	0.650	0.657	0.663	0.663	0.668	0.673	0.668	0.673	0.668	0.673	0.668	0.673	0.668	0.673	0.668	0.673		
46.63	8.980	120	26.6650	19.9414	16.1775	13.7275	11.9871	10.6778	0.630	0.632	0.636	0.636	0.642	0.648	0.636	0.642	0.636	0.642	0.636	0.642	0.636	0.642	0.636	0.642		

Eb = 90 ksi
6 inches

Eac (ksi)	hEP (inches)	tp (deg. F.)	d0 (mils)	d0 (mils)	d0 (mils)	d0 (mils)	TAF	TAF	TAF	TAF	TAF	
2293.46	7.359	30	9.4588	7.1986	5.9294	5.0990	4.5065	4.0589	3.329	1.324	1.320	1.318
2074.71	7.405	35	9.7146	7.3933	6.0882	5.2349	4.6262	4.1662	1.294	1.292	1.288	1.284
1850.36	7.460	40	10.0128	7.6189	6.2730	5.3931	4.7654	4.2910	1.256	1.253	1.250	1.248
1627.24	7.524	45	10.3553	7.8779	6.4851	5.5746	4.9250	4.4341	1.214	1.212	1.211	1.207
1411.23	7.598	50	10.7444	8.1720	6.7258	5.7804	5.1059	4.5962	1.170	1.169	1.167	1.164
1207.09	7.684	55	11.1826	8.5030	6.9965	6.0117	5.3091	4.7782	1.124	1.123	1.122	1.121
1018.40	7.782	60	11.6727	8.8727	7.2986	6.2697	5.5356	4.9810	1.077	1.076	1.075	1.074
847.58	7.894	65	12.2173	9.2832	7.6338	6.5556	5.7864	5.2054	1.029	1.029	1.028	1.028
754.20	7.969	68	12.5716	9.5500	7.8514	6.7411	5.9491	5.3508	1.000	1.000	1.000	1.000
695.91	8.023	70	12.8193	9.7364	8.0034	6.8706	6.0626	5.4522	0.981	0.981	0.981	0.981
563.73	8.170	75	13.4813	10.2341	8.4088	7.2158	6.3648	5.7221	0.933	0.934	0.934	0.935
450.57	8.338	80	14.2058	10.7779	8.8511	7.5919	6.6938	6.0155	0.886	0.887	0.888	0.890
355.36	8.531	85	14.9849	11.3690	9.3312	7.9996	7.0499	6.3327	0.838	0.840	0.843	0.845
276.56	8.751	90	15.8499	12.0082	9.8494	8.4388	7.4330	6.6735	0.793	0.795	0.797	0.799
212.41	9.004	95	16.7715	12.6955	10.4054	8.9093	7.8426	7.0373	0.750	0.752	0.755	0.757
161.00	9.295	100	17.7592	13.4302	10.9983	9.4100	8.2777	7.4231	0.708	0.711	0.714	0.716
120.45	9.630	105	18.8111	14.2103	11.6263	9.9390	8.7365	7.8291	0.688	0.672	0.678	0.683
88.94	10.016	110	19.9241	15.0329	12.2866	10.4938	9.2166	8.2530	0.631	0.635	0.642	0.648
64.82	10.462	115	21.0932	15.8937	12.9754	11.0710	9.7148	8.6920	0.596	0.601	0.605	0.616
46.63	10.980	120	22.3116	16.7873	13.6881	11.6665	10.2274	9.1426	0.563	0.569	0.574	0.585

E_b = 90 ksi
hac = 8 inches

| E_ac (ksi) | hEP (inches) | I_p (deg. F) | M₁ K₁ | M₂ K₂ | M₃ K₃ | M₄ K₄ | M₅ K₅ | M₆ K₆ | M₇ K₇ | M₈ K₈ | M₉ K₉ | M₁₀ K₁₀ | M₁₁ K₁₁ | M₁₂ K₁₂ | M₁₃ K₁₃ | M₁₄ K₁₄ | M₁₅ K₁₅ | M₁₆ K₁₆ | M₁₇ K₁₇ | M₁₈ K₁₈ | M₁₉ K₁₉ | M₂₀ K₂₀ | M₂₁ K₂₁ | M₂₂ K₂₂ | M₂₃ K₂₃ | M₂₄ K₂₄ | M₂₅ K₂₅ | M₂₆ K₂₆ | M₂₇ K₂₇ | M₂₈ K₂₈ | M₂₉ K₂₉ | M₃₀ K₃₀ | M₃₁ K₃₁ | M₃₂ K₃₂ | M₃₃ K₃₃ | M₃₄ K₃₄ | M₃₅ K₃₅ | M₃₆ K₃₆ | M₃₇ K₃₇ | M₃₈ K₃₈ | M₃₉ K₃₉ | M₄₀ K₄₀ | M₄₁ K₄₁ | M₄₂ K₄₂ | M₄₃ K₄₃ | M₄₄ K₄₄ | M₄₅ K₄₅ | M₄₆ K₄₆ | M₄₇ K₄₇ | M₄₈ K₄₈ | M₄₉ K₄₉ | M₅₀ K₅₀ | M₅₁ K₅₁ | M₅₂ K₅₂ | M₅₃ K₅₃ | M₅₄ K₅₄ | M₅₅ K₅₅ | M₅₆ K₅₆ | M₅₇ K₅₇ | M₅₈ K₅₈ | M₅₉ K₅₉ | M₆₀ K₆₀ | M₆₁ K₆₁ | M₆₂ K₆₂ | M₆₃ K₆₃ | M₆₄ K₆₄ | M₆₅ K₆₅ | M₆₆ K₆₆ | M₆₇ K₆₇ | M₆₈ K₆₈ | M₆₉ K₆₉ | M₇₀ K₇₀ | M₇₁ K₇₁ | M₇₂ K₇₂ | M₇₃ K₇₃ | M₇₄ K₇₄ | M₇₅ K₇₅ | M₇₆ K₇₆ | M₇₇ K₇₇ | M₇₈ K₇₈ | M₇₉ K₇₉ | M₈₀ K₈₀ | M₈₁ K₈₁ | M₈₂ K₈₂ | M₈₃ K₈₃ | M₈₄ K₈₄ | M₈₅ K₈₅ | M₈₆ K₈₆ | M₈₇ K₈₇ | M₈₈ K₈₈ | M₈₉ K₈₉ | M₉₀ K₉₀ | M₉₁ K₉₁ | M₉₂ K₉₂ | M₉₃ K₉₃ | M₉₄ K₉₄ | M₉₅ K₉₅ | M₉₆ K₉₆ | M₉₇ K₉₇ | M₉₈ K₉₈ | M₉₉ K₉₉ | M₁₀₀ K₁₀₀ | M₁₀₁ K₁₀₁ | M₁₀₂ K₁₀₂ | M₁₀₃ K₁₀₃ | M₁₀₄ K₁₀₄ | M₁₀₅ K₁₀₅ | M₁₀₆ K₁₀₆ | M₁₀₇ K₁₀₇ | M₁₀₈ K₁₀₈ | M₁₀₉ K₁₀₉ | M₁₁₀ K₁₁₀ | M₁₁₁ K₁₁₁ | M₁₁₂ K₁₁₂ | M₁₁₃ K₁₁₃ | M₁₁₄ K₁₁₄ | M₁₁₅ K₁₁₅ | M₁₁₆ K₁₁₆ | M₁₁₇ K₁₁₇ | M₁₁₈ K₁₁₈ | M₁₁₉ K₁₁₉ | M₁₂₀ K₁₂₀ | M₁₂₁ K₁₂₁ | M₁₂₂ K₁₂₂ | M₁₂₃ K₁₂₃ | M₁₂₄ K₁₂₄ | M₁₂₅ K₁₂₅ | M₁₂₆ K₁₂₆ | M₁₂₇ K₁₂₇ | M₁₂₈ K₁₂₈ | M₁₂₉ K₁₂₉ | M₁₃₀ K₁₃₀ | M₁₃₁ K₁₃₁ | M₁₃₂ K₁₃₂ | M₁₃₃ K₁₃₃ | M₁₃₄ K₁₃₄ | M₁₃₅ K₁₃₅ | M₁₃₆ K₁₃₆ | M₁₃₇ K₁₃₇ | M₁₃₈ K₁₃₈ | M₁₃₉ K₁₃₉ | M₁₄₀ K₁₄₀ | M₁₄₁ K₁₄₁ | M₁₄₂ K₁₄₂ | M₁₄₃ K₁₄₃ | M₁₄₄ K₁₄₄ | M₁₄₅ K₁₄₅ | M₁₄₆ K₁₄₆ | M₁₄₇ K₁₄₇ | M₁₄₈ K₁₄₈ | M₁₄₉ K₁₄₉ | M₁₅₀ K₁₅₀ | M₁₅₁ K₁₅₁ | M₁₅₂ K₁₅₂ | M₁₅₃ K₁₅₃ | M₁₅₄ K₁₅₄ | M₁₅₅ K₁₅₅ | M₁₅₆ K₁₅₆ | M₁₅₇ K₁₅₇ | M₁₅₈ K₁₅₈ | M₁₅₉ K₁₅₉ | M₁₆₀ K₁₆₀ | M₁₆₁ K₁₆₁ | M₁₆₂ K₁₆₂ | M₁₆₃ K₁₆₃ | M₁₆₄ K₁₆₄ | M₁₆₅ K₁₆₅ | M₁₆₆ K₁₆₆ | M₁₆₇ K₁₆₇ | M₁₆₈ K₁₆₈ | M₁₆₉ K₁₆₉ | M₁₇₀ K₁₇₀ | M₁₇₁ K₁₇₁ | M₁₇₂ K₁₇₂ | M₁₇₃ K₁₇₃ | M₁₇₄ K₁₇₄ | M₁₇₅ K₁₇₅ | M₁₇₆ K₁₇₆ | M₁₇₇ K₁₇₇ | M₁₇₈ K₁₇₈ | M₁₇₉ K₁₇₉ | M₁₈₀ K₁₈₀ | M₁₈₁ K₁₈₁ | M₁₈₂ K₁₈₂ | M₁₈₃ K₁₈₃ | M₁₈₄ K₁₈₄ | M₁₈₅ 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E _b =	90 ksi
hac =	10 inches

E _a (ksi)	hEP (inches)	tp (deg. F)	M ₁			M ₂			M ₃			M ₄			M ₅			M ₆		
			d ₀	(mils)	d ₀	(mils)	d ₀	(mils)	d ₀	(mils)	d ₀	(mils)	d ₀	(mils)	d ₀	(mils)	d ₀	(mils)	d ₀	(mils)
2293.46	11.359	30	6.1583	4.6945	3.8715	3.3334	2.9494	2.6592	2.7371	1.331	1.330	1.331	1.369	1.368	1.367	1.366	1.365	1.366	1.365	
2074.71	11.405	35	6.3405	4.8331	3.9855	3.4313	3.0359	2.8283	3.1373	1.286	1.287	1.286	1.285	1.329	1.328	1.327	1.326	1.327	1.326	
1850.36	11.460	40	6.5539	4.9954	4.1190	3.5461	3.6787	3.2543	2.9337	1.241	1.240	1.240	1.240	1.239	1.238	1.237	1.236	1.237	1.236	
1627.24	11.524	45	6.8007	5.1830	4.2734	4.4499	3.8303	3.3882	3.0541	1.192	1.191	1.191	1.191	1.190	1.190	1.190	1.189	1.189	1.189	
1411.23	11.598	50	7.0832	5.3976	4.4499	4.6503	4.0024	3.5400	3.1907	1.140	1.140	1.140	1.140	1.139	1.139	1.139	1.138	1.138	1.138	
1207.09	11.684	55	7.4041	5.6414	4.8765	4.1965	3.7113	3.3446	3.0765	1.087	1.087	1.087	1.086	1.086	1.086	1.086	1.085	1.086	1.085	
1018.40	11.782	60	7.7664	5.9166	5.1303	4.4142	3.9033	3.5173	3.0000	1.033	1.033	1.033	1.032	1.032	1.032	1.032	1.032	1.032	1.032	
847.58	11.894	65	8.1735	6.2256	5.2969	4.5571	4.0293	3.6304	3.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
754.20	11.969	68	8.4410	6.4284	5.4141	4.6576	4.1178	3.7099	3.0000	0.978	0.978	0.978	0.978	0.978	0.978	0.978	0.978	0.978	0.978	
695.91	12.023	70	8.6291	6.5712	5.7300	4.9283	4.3564	3.9242	3.0000	0.924	0.924	0.924	0.924	0.924	0.924	0.924	0.925	0.925	0.925	
563.73	12.170	75	9.1372	6.9562	5.7300	4.9283	4.3564	3.9242	3.0000	0.871	0.871	0.871	0.871	0.871	0.871	0.871	0.872	0.872	0.872	
450.57	12.338	80	9.7017	7.3838	6.0806	5.2286	4.6207	4.1614	3.0000	0.818	0.818	0.818	0.818	0.818	0.818	0.818	0.820	0.820	0.821	
355.36	12.531	85	10.3272	7.8570	6.4682	5.5603	4.9126	4.4232	3.0000	0.766	0.766	0.766	0.766	0.766	0.766	0.766	0.770	0.770	0.771	
276.56	12.751	90	11.0177	8.3789	6.8954	5.9255	5.2336	4.7109	3.0000	0.717	0.717	0.717	0.717	0.717	0.717	0.717	0.720	0.720	0.721	
212.41	13.004	95	11.7776	8.9524	7.3642	6.3259	5.5653	5.0257	3.0000	0.669	0.669	0.669	0.669	0.669	0.669	0.669	0.675	0.675	0.676	
161.00	13.295	100	12.6107	9.5803	7.8767	6.7631	5.9688	5.3687	3.0000	0.624	0.624	0.624	0.624	0.624	0.624	0.624	0.631	0.631	0.632	
120.45	13.630	105	13.5203	10.2644	8.4343	7.2381	6.3849	5.7404	3.0000	0.584	0.584	0.584	0.584	0.584	0.584	0.584	0.591	0.591	0.591	
88.94	14.016	110	14.5087	11.0064	9.0379	7.7515	6.8340	6.1411	3.0000	0.544	0.544	0.544	0.544	0.544	0.544	0.544	0.551	0.551	0.553	
64.82	14.462	115	15.5774	11.8067	9.6877	8.3031	7.3157	6.5701	3.0000	0.505	0.505	0.505	0.505	0.505	0.505	0.505	0.513	0.513	0.515	
46.63	14.980	120	16.7258	12.6644	10.3825	8.8917	7.8288	7.0264	3.0000	0.517	0.517	0.517	0.517	0.517	0.517	0.517	0.515	0.515	0.517	

E_b = 90 ksi
hac = 12 inches

E _{ac} (ksi)	hEp (inches)	Ip (deg. F)	W ₁₅			W ₁₆			W ₁₇			W ₁₈			W ₁₉			W ₂₀		
			10	15	20	15	20	25	15	20	25	15	20	25	15	20	25	15	20	25
2293.46	13.359	30	5.2416	3.9970	3.2971	2.8395	2.5130	2.2663	1.382	1.380	1.380	1.340	1.340	1.340	1.339	1.339	1.339	1.338	1.338	1.338
2074.71	13.405	35	5.4002	4.1177	3.3966	2.9251	2.5886	2.3344	1.342	1.341	1.341	1.296	1.296	1.296	1.294	1.294	1.294	1.294	1.294	1.294
1850.36	13.460	40	5.5864	4.2594	3.5132	3.0254	2.6773	2.4143	1.297	1.297	1.297	1.248	1.248	1.248	1.247	1.247	1.247	1.246	1.246	1.246
1627.24	13.524	45	5.8020	4.4235	3.6484	3.1416	2.7800	2.5067	1.249	1.249	1.249	1.198	1.198	1.198	1.197	1.197	1.197	1.196	1.196	1.196
1411.23	13.598	50	6.0493	4.6116	3.8033	3.2748	2.8976	2.6127	1.198	1.197	1.197	1.144	1.144	1.144	1.143	1.143	1.143	1.143	1.143	1.143
1207.09	13.684	55	6.3309	4.8258	3.9796	3.4263	3.0315	2.7332	1.144	1.144	1.144	1.090	1.090	1.090	1.089	1.089	1.089	1.089	1.089	1.089
1018.40	13.782	60	6.6497	5.0682	4.1790	3.5977	3.1828	2.8694	1.090	1.090	1.090	1.034	1.034	1.034	1.033	1.033	1.033	1.033	1.033	1.033
847.58	13.894	65	7.0090	5.3413	4.4037	3.7906	3.3532	3.0227	1.034	1.034	1.034	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
754.20	13.969	68	7.2456	5.5211	4.5515	3.9176	3.4653	3.1235	1.000	1.000	1.000	0.977	0.977	0.977	0.976	0.976	0.976	0.976	0.976	0.976
695.91	14.023	70	7.4124	5.6479	4.6557	4.0070	3.5442	3.1945	0.977	0.978	0.978	0.922	0.922	0.922	0.922	0.922	0.922	0.922	0.922	0.922
563.73	14.170	75	7.8640	5.9908	4.9375	4.2489	3.7576	3.3864	0.921	0.921	0.921	0.866	0.866	0.866	0.867	0.867	0.867	0.867	0.867	0.867
450.57	14.338	80	8.3680	6.3733	5.2517	4.5185	3.9953	3.6000	0.811	0.811	0.811	0.759	0.759	0.759	0.761	0.761	0.761	0.761	0.761	0.761
355.36	14.531	85	8.9291	6.7988	5.6010	4.8179	4.2592	3.8370	0.759	0.759	0.759	0.708	0.708	0.708	0.711	0.711	0.711	0.711	0.711	0.711
276.56	14.751	90	9.5520	7.2707	5.9881	5.1496	4.5513	4.0992	0.759	0.759	0.759	0.659	0.659	0.659	0.661	0.661	0.661	0.661	0.661	0.661
212.41	15.004	95	10.2417	7.7927	6.4158	5.5157	4.8735	4.3883	0.707	0.707	0.707	0.613	0.613	0.613	0.615	0.615	0.615	0.615	0.615	0.615
161.00	15.295	100	11.0029	8.3681	6.8869	5.9186	5.2277	4.7058	0.659	0.659	0.659	0.530	0.530	0.530	0.571	0.571	0.571	0.571	0.571	0.571
120.45	15.630	105	11.8401	9.0002	7.4037	6.3600	5.6155	5.0530	0.612	0.612	0.612	0.568	0.568	0.568	0.532	0.532	0.532	0.532	0.532	0.532
88.94	16.016	110	12.7576	9.6917	7.9682	6.8416	6.0380	5.4310	0.570	0.570	0.570	0.527	0.527	0.527	0.492	0.492	0.492	0.492	0.492	0.492
64.82	16.462	115	13.7585	10.4446	8.5819	7.3644	6.4961	5.8402	0.529	0.529	0.529	0.488	0.488	0.488	0.494	0.494	0.494	0.494	0.494	0.494
46.63	16.980	120	14.8450	11.2601	9.2454	7.9287	6.9897	6.2806	0.488	0.488	0.488	0.492	0.492	0.492	0.496	0.496	0.496	0.496	0.496	0.496

TEMPERATURE ADJUSTMENT FACTOR CURVE
REGRESSION EQUATIONS

2" depth: $TAF = 1.358192 - 0.005227T(^{\circ}F)$
 $CC = -0.996296$

4" depth: $TAF = 1.522908 - 0.007653T(^{\circ}F)$
 $CC = -0.997893$

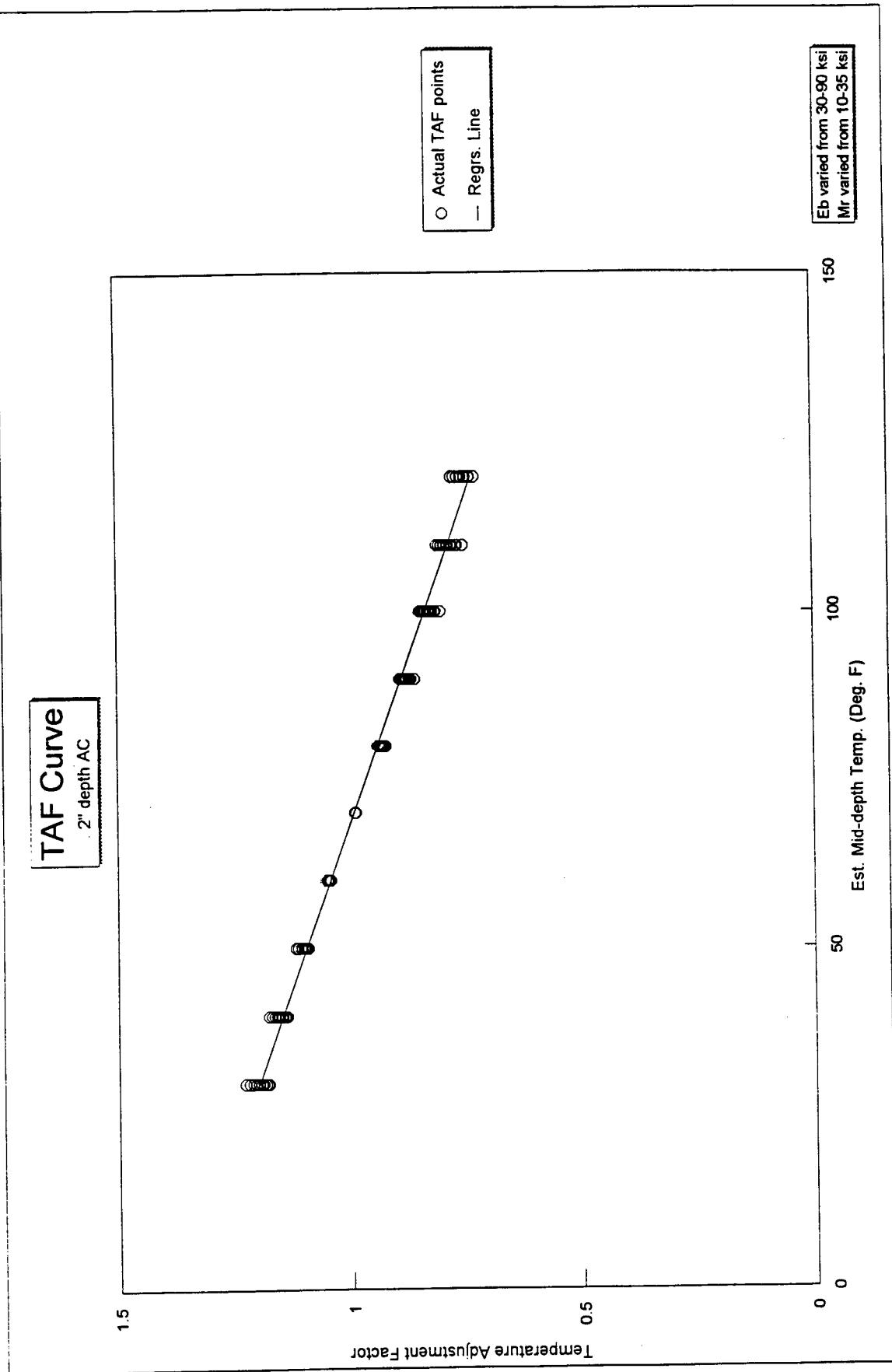
6" depth: $TAF = 1.605983 - 0.00888T(^{\circ}F)$
 $CC = -0.998475$

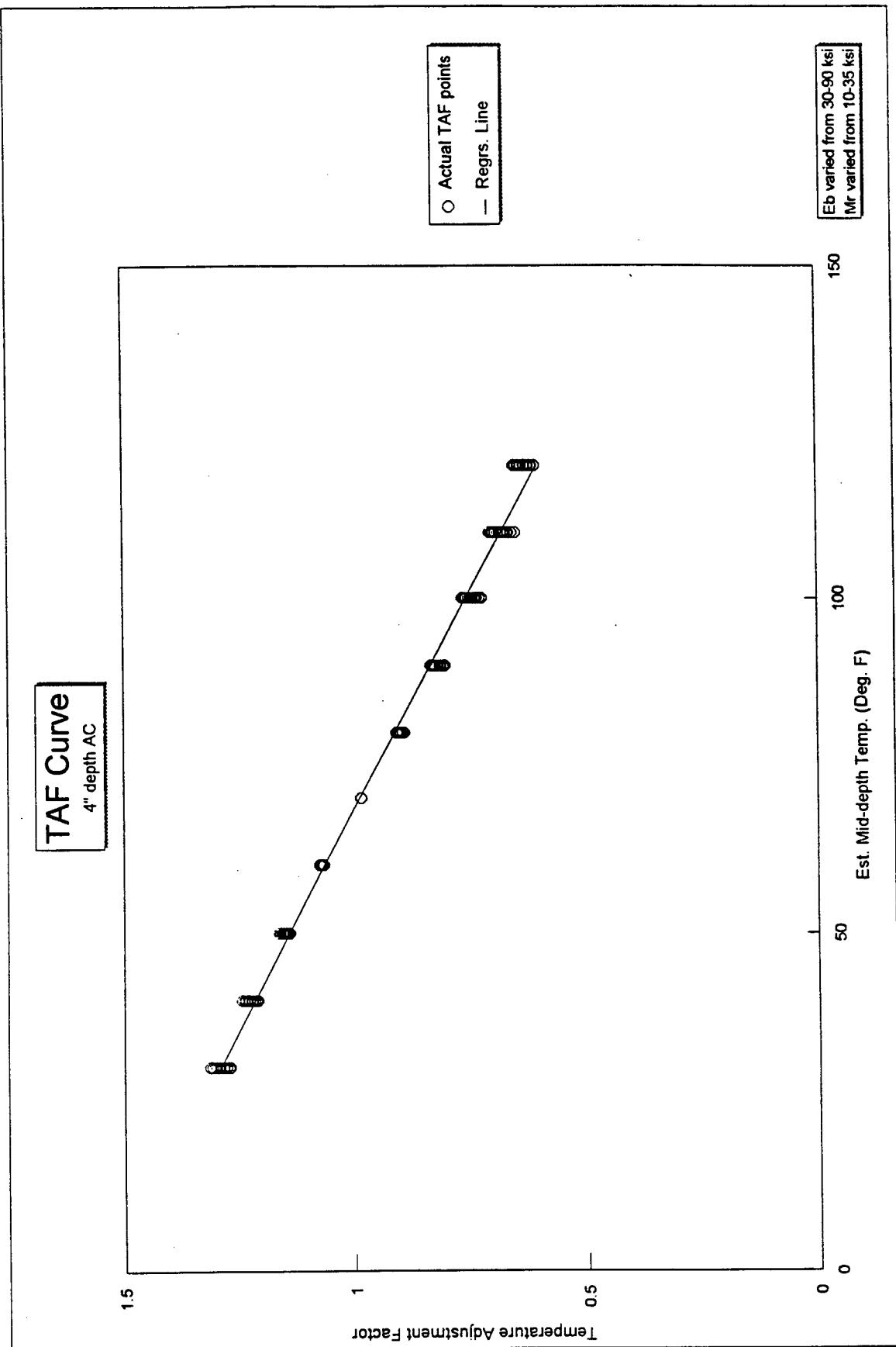
8" depth: $TAF = 1.654622 - 0.00961T(^{\circ}F)$
 $CC = -0.99875$

10" depth: $TAF = 1.686027 - 0.01007T(^{\circ}F)$
 $CC = -0.998884$

12" depth: $TAF = 1.707725 - 0.0104T(^{\circ}F)$
 $CC = -0.998995$

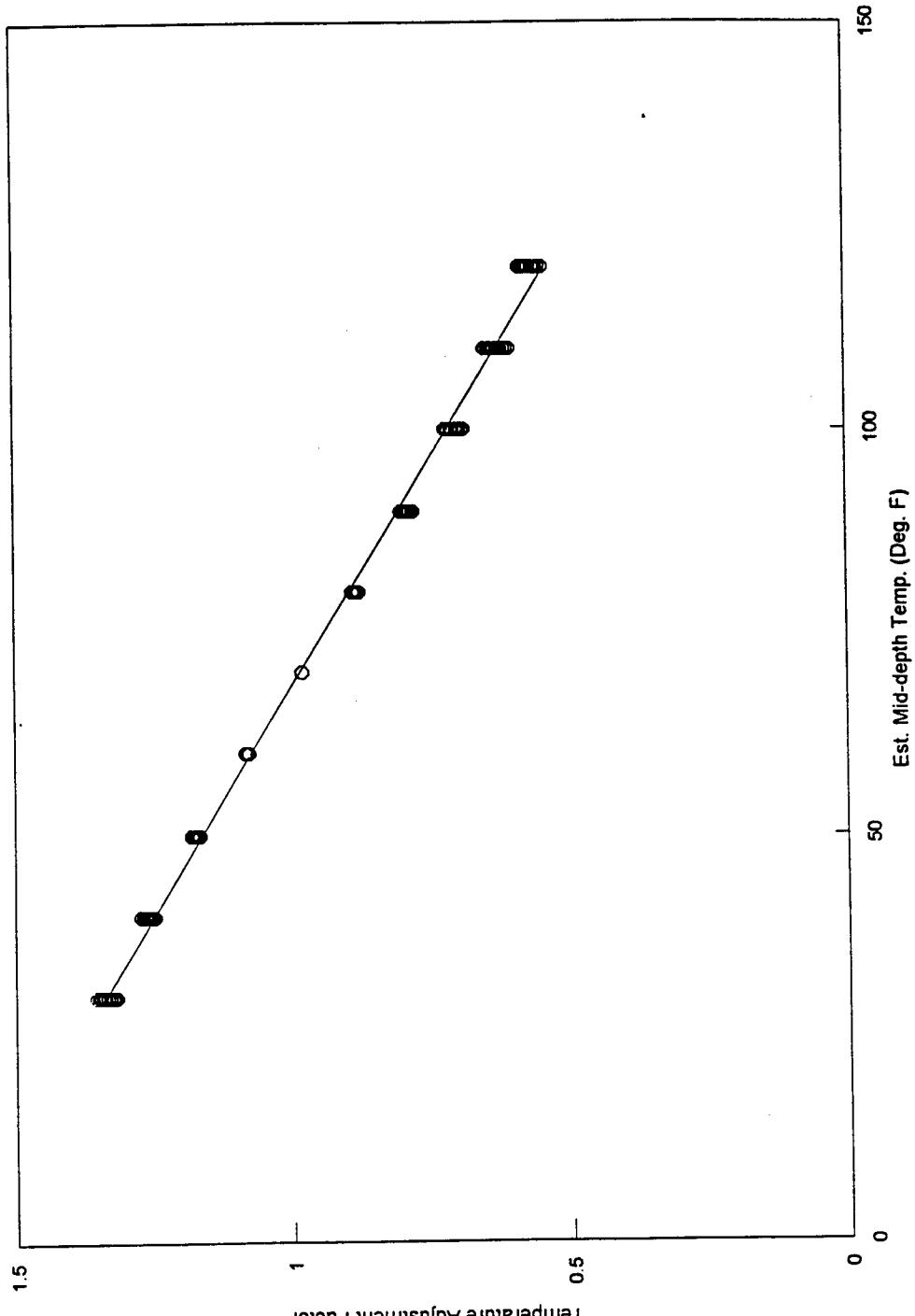
where CC is the correlation coefficient



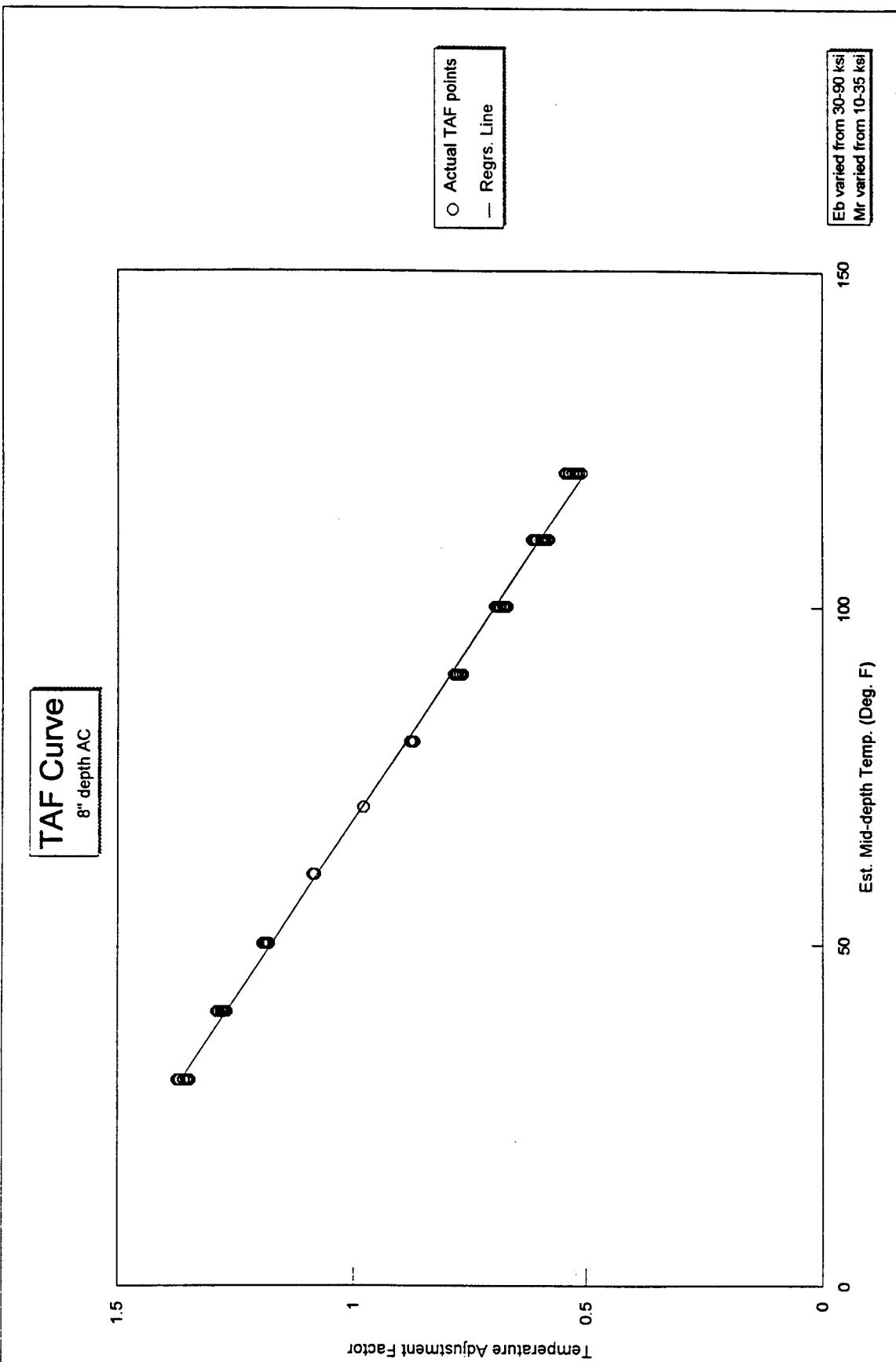


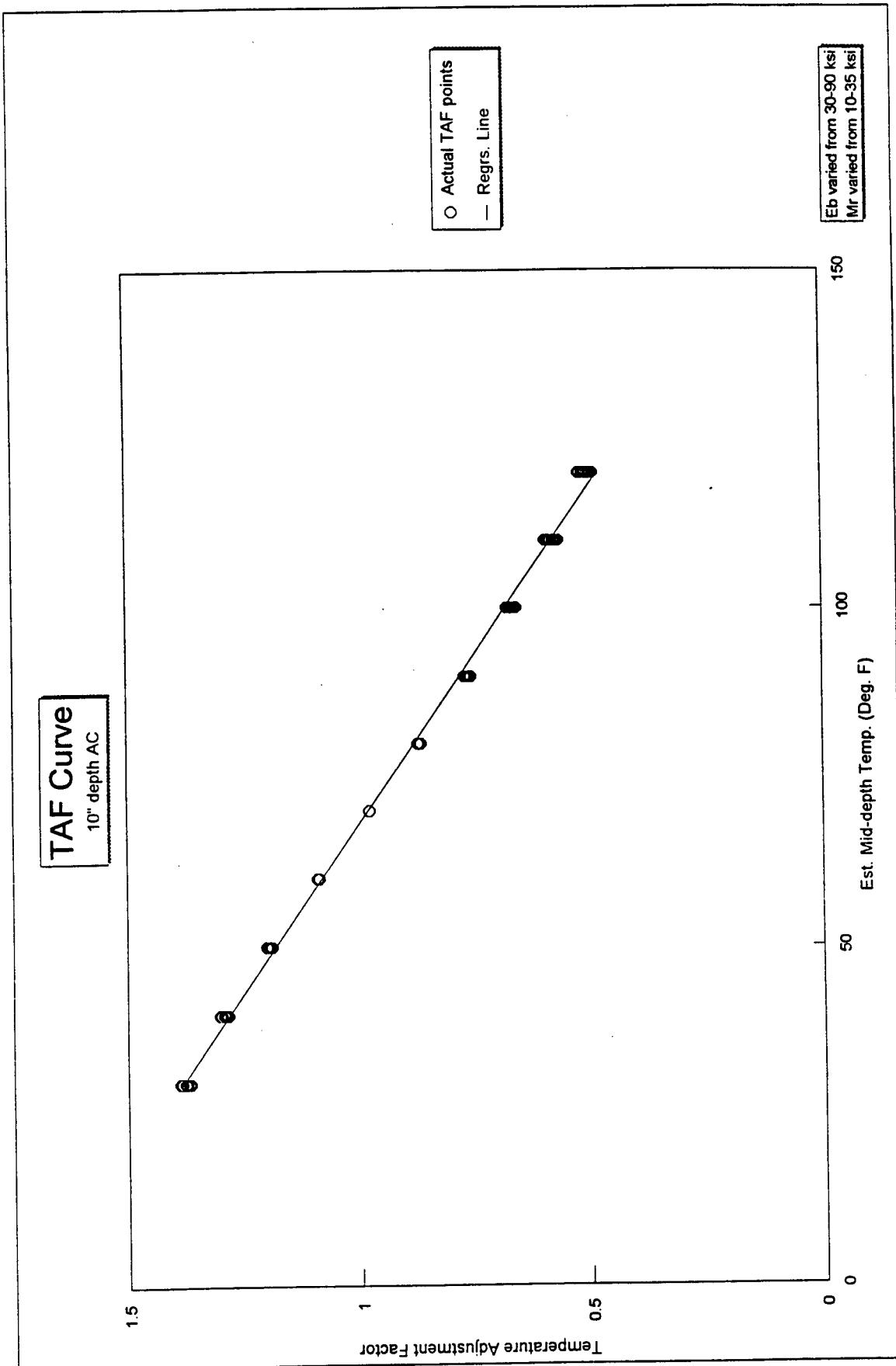
TAF Curve

6" depth AC

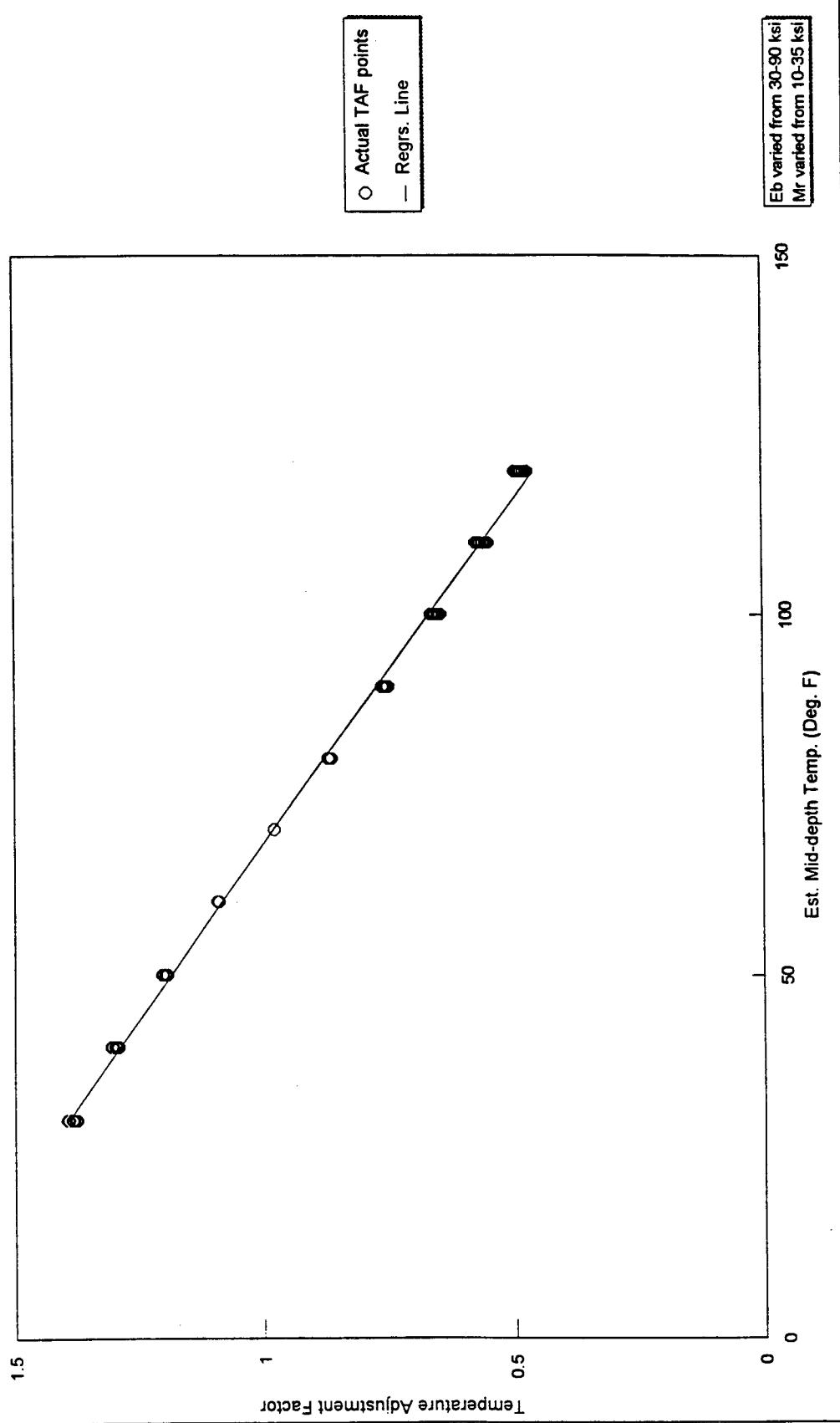


Temperature Adjustment Factor

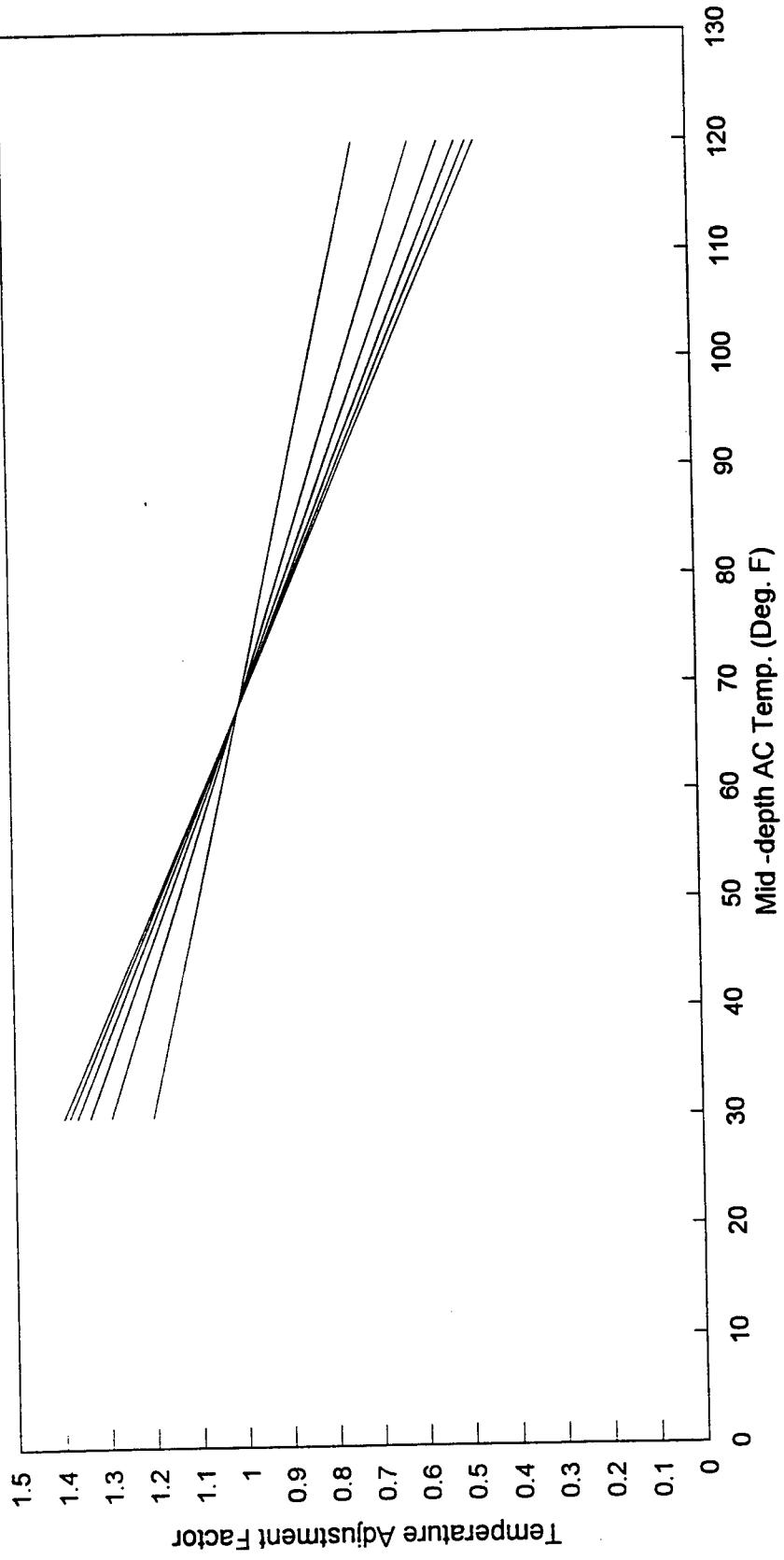




TAF Curve
12" depth AC



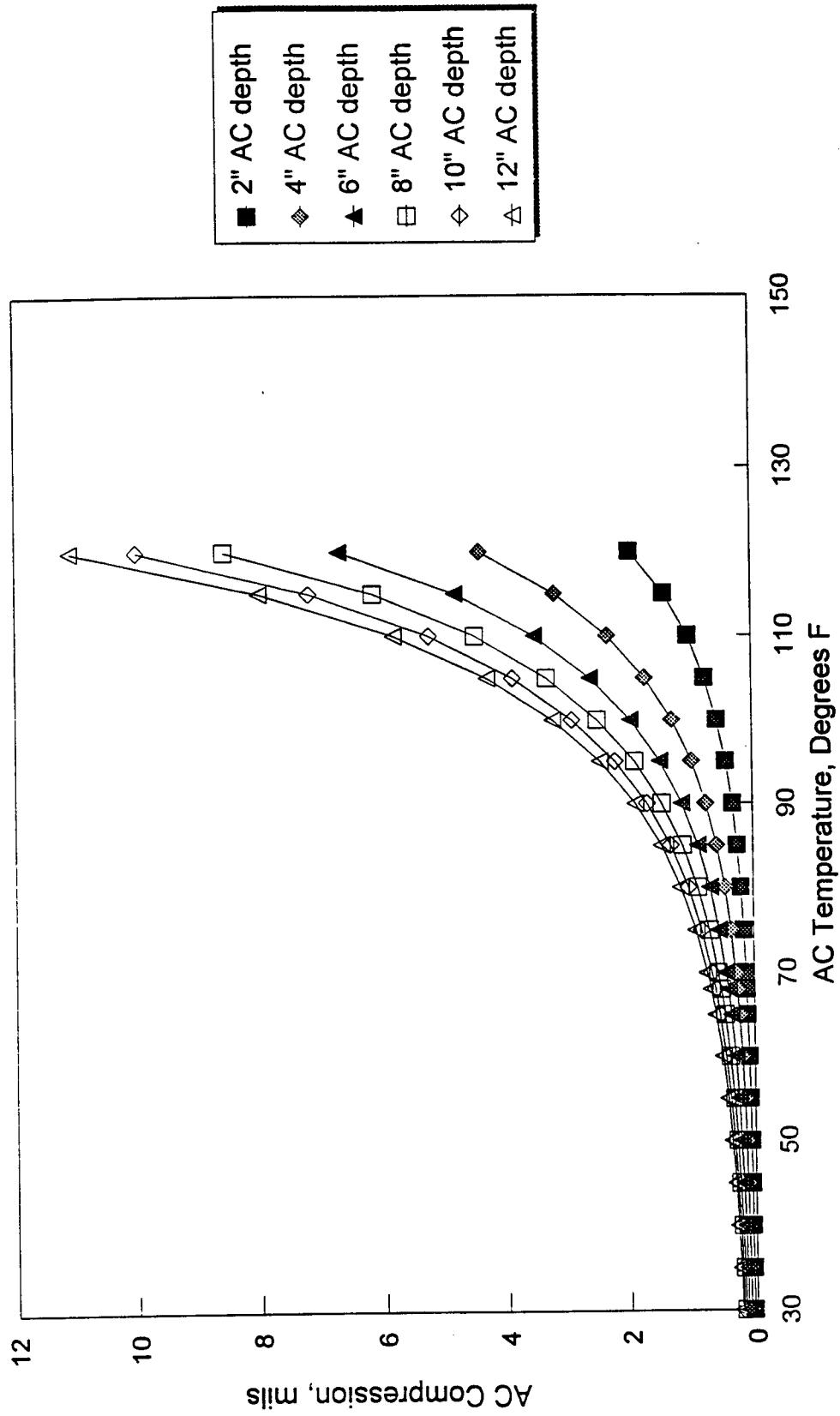
Temperature Adjustment Factor Curve
for Various AC Thicknesses



Bousinessq Method
for Center of Plate AC Compression

(t/p) (deg F)	E(ac) (psi)	AC Compression at Depth						TAF at Depth					
		2 inches (mils)	4 inches (mils)	6 inches (mils)	8 inches (mils)	10 inches (mils)	12 inches (mils)	2 inches	4 inches	6 inches	8 inches	10 inches	12 inches
0	3.065E+06	0.0295	0.0670	0.1018	0.1300	0.1518	0.1687	4.064143	4.064143	4.064143	4.064143	4.064143	4.064143
5	3.037E+06	0.0298	0.0676	0.1027	0.1312	0.1532	0.1703	4.026885	4.026885	4.026885	4.026885	4.026885	4.026885
10	2.960E+06	0.0305	0.0694	0.1054	0.1346	0.1572	0.1747	3.924423	3.924423	3.924423	3.924423	3.924423	3.924423
15	2.840E+06	0.0318	0.0723	0.1099	0.1403	0.1639	0.1821	3.765323	3.765323	3.765323	3.765323	3.765323	3.765323
20	2.684E+06	0.0337	0.0765	0.1163	0.1485	0.1734	0.1927	3.55842	3.55842	3.55842	3.55842	3.55842	3.55842
25	2.499E+06	0.0362	0.0822	0.1249	0.1594	0.1862	0.2070	3.313464	3.313464	3.313464	3.313464	3.313464	3.313464
30	2.293E+06	0.0394	0.0895	0.1361	0.1737	0.2029	0.2255	3.040775	3.040775	3.040775	3.040775	3.040775	3.040775
35	2.075E+06	0.0436	0.0990	0.1504	0.1920	0.2243	0.2493	2.75074	2.75074	2.75074	2.75074	2.75074	2.75074
40	1.850E+06	0.0488	0.1110	0.1687	0.2153	0.2515	0.2795	2.453292	2.453292	2.453292	2.453292	2.453292	2.453292
45	1.627E+06	0.0555	0.1262	0.1918	0.2448	0.2860	0.3178	2.157471	2.157471	2.157471	2.157471	2.157471	2.157471
50	1.411E+06	0.0640	0.1455	0.2211	0.2823	0.3298	0.3665	1.87107	1.87107	1.87107	1.87107	1.87107	1.87107
55	1.207E+06	0.0749	0.1701	0.2585	0.3301	0.3855	0.4285	1.600412	1.600412	1.600412	1.600412	1.600412	1.600412
60	1.018E+06	0.0887	0.2016	0.3064	0.3912	0.4570	0.5079	1.350246	1.350246	1.350246	1.350246	1.350246	1.350246
65	8.476E+05	0.1066	0.2423	0.3682	0.4701	0.5491	0.6102	1.123757	1.123757	1.123757	1.123757	1.123757	1.123757
68	7.542E+05	0.1198	0.2722	0.4138	0.5282	0.6170	0.6858	1	1	1	1	1	1
70	6.959E+05	0.1299	0.2951	0.4484	0.5725	0.6687	0.7432	0.922666	0.922666	0.922666	0.922666	0.922666	0.922666
75	5.637E+05	0.1603	0.3642	0.5536	0.7068	0.8255	0.9175	0.747417	0.747417	0.747417	0.747417	0.747417	0.747417
80	4.506E+05	0.2006	0.4557	0.6926	0.8843	1.0329	1.1479	0.597389	0.597389	0.597389	0.597389	0.597389	0.597389
85	3.554E+05	0.2543	0.5778	0.8782	1.1212	1.3096	1.4555	0.471145	0.471145	0.471145	0.471145	0.471145	0.471145
90	2.766E+05	0.3268	0.7424	1.1284	1.4406	1.6828	1.8702	0.366676	0.366676	0.366676	0.366676	0.366676	0.366676
95	2.124E+05	0.4255	0.9667	1.4692	1.8757	2.1910	2.4350	0.281621	0.281621	0.281621	0.281621	0.281621	0.281621
100	1.610E+05	0.5613	1.2753	1.9383	2.4746	2.8906	3.2125	0.213464	0.213464	0.213464	0.213464	0.213464	0.213464
105	1.204E+05	0.7503	1.7048	2.5910	3.3079	3.8639	4.2942	0.159692	0.159692	0.159692	0.159692	0.159692	0.159692
110	8.893E+04	1.0162	2.3088	3.5090	4.4800	5.2329	5.8157	0.117914	0.117914	0.117914	0.117914	0.117914	0.117914
115	6.482E+04	1.3942	3.1679	4.8147	6.1469	7.1800	7.9796	0.085938	0.085938	0.085938	0.085938	0.085938	0.085938
120	4.663E+04	1.9380	4.4034	6.6925	8.5443	9.9803	11.0919	0.061825	0.061825	0.061825	0.061825	0.061825	0.061825
125	3.311E+04	2.7290	6.2006	9.4240	12.0316	14.0537	15.6189	0.043905	0.043905	0.043905	0.043905	0.043905	0.043905
130	2.322E+04	3.8928	8.8447	13.4427	17.1623	20.0466	22.2793	0.03078	0.03078	0.03078	0.03078	0.03078	0.03078
135	1.607E+04	5.6247	12.7799	19.4236	24.7980	28.9657	32.1917	0.021302	0.021302	0.021302	0.021302	0.021302	0.021302

Boussinesq AC Compression
at 9 kips and $a = 5.9"$



APPENDIX D

**Data and Graphs Relating to
the Comparison of Methods Used to Find Layer Strength Moduli**

MANUALLY BACKCALCULATED PROCEDURE FOR FINDING E_p AND M_r

Determination of Subgrade Modulus for SN_{eff} Determination

1993 AASHTO Design Guide of Pavement Structures

Choose the radial distance from the load in inches, r . The first value can be approximated as 1.5 times the total depth of the pavement.

Find the deflection value, D_r , corresponding to the chosen radial distance. The value is interpolated from the FWD deflection data gathered.

Find the subgrade resilient modulus:

$$M_r = \frac{P(1-u^2)}{pi * r * D_r}$$

where P = load in lbs. (9000 lbs)

u = Poisson's ratio (0.5)

D_r = deflection at distance r from the applied load in inches (varies with r)

r = radial distance from load in inches (varied)

Find E_p by trial and error from the following equation:

$$d_0 = 1.5pa \left[\frac{1}{M_r \sqrt{1 + \left(\frac{D}{a} \sqrt[3]{\frac{E_p}{M_r}} \right)^2}} + \frac{\left[1 - \frac{1}{\sqrt{1 + \left(\frac{D}{a} \right)^2}} \right]}{E_p} \right]$$

where D = total depth of pavement in inches (varied for different pavements)

a = plate radius in inches (5.9 inches)

E_p = strength modulus of the pavement in psi

M_r = resilient modulus of subgrade in psi (as found above)

p = contact pressure in psi (9000 lbs./plate area = 82.3 psi)

d_0 = deflection value at $r=0$ in inches (varies)

The radius value used (r) should be greater than or equal to seventy percent of the effective radius a_e of the stress bulb at the subgrade/pavement interface. With the subgrade Poisson's ratio equal to 0.5, the equation for effective radius is:

$$a_e = \sqrt{a^2 + \left(D \sqrt[3]{\frac{E_p}{M_r}} \right)^2}$$

$$r \geq 0.7a_e$$

If the r value used is less than $0.7a_e$ or a value of r that is closer to the calculated $0.7a_e$ can be used, then another value for r is chosen and the procedure is started all over again until r is greater than or equal to $0.7a_e$. The final trial yields the values for E_p and M_r for that particular FWD deflection reading..

SAMPLE CALCULATION

Test Site: MT1

Date of FWD deflection data collection: 2/11/92

$$D = 11"$$

$$a = 5.9"$$

$$P = 9000 \text{ lbs.}$$

$$\text{estimated mid - depth temperature} = 38.7^\circ$$

FWD Deflection Data:

Mean Measured Deflection: (mils)

$R_0 = 0"$	$R_1 = 8"$	$R_2 = 12"$	$R_3 = 24"$	$R_4 = 36"$	$R_5 = 48"$	$R_6 = 72"$
6.75	6.13	5.61	4.11	2.86	2.03	1.16

$E_p, M_r, \text{ CALCULATION}$

*initial estimate of $r = 1.5D$ or $1.5*11 = 16.5"$*

$$r = 16.5"$$

$$D_r, \text{ found by interpolation. } \frac{24 - 16.5}{24 - 12} = \frac{4.11 - D_r}{4.11 - 5.61} \quad D_r = 5.05 \text{ mils}$$

$$M_r = \frac{P(1-u^2)}{\pi * r * D_r} = \frac{9000(1-0.5^2)}{3.14 * 16.5 * 5.05 * 10^{-3}} = 25,785.68 \text{ psi}$$

$$6.75 * 10^{-3} = 1.5 * 82.3 * 5.9 \left[\frac{1}{25,785.68 \sqrt{1 + \left(\frac{11}{5.9} \sqrt[3]{\frac{E_p}{25,785.68}} \right)^2}} + \frac{1}{E_p} \left(1 - \frac{1}{\sqrt{1 + \left(\frac{11}{5.9} \right)^2}} \right) \right]$$

E_p solved for by inputted solver programon HP42S Scientific Calculator

$$E_p = 421,796.79 \text{ psi}$$

$$a_e = \sqrt{5.9^2 + \left(11 * \sqrt[3]{\frac{421.796.79}{25,785.68}} \right)^2} = 28.54"$$

$$0.7a_e = 19.98"$$

$0.7a_e > r$ therefore, no good

new $r = 21"$

$$\frac{24 - 21}{24 - 12} = \frac{4.11 - D_r}{4.11 - 5.61} \quad D_r = 4.49 \text{ mils}$$

$$M_r = \frac{9000(1 - 0.5^2)}{3.14 * 21 * 4.49 * 10^{-3}} = 22,787.06 \text{ psi}$$

$$6.75 * 10^{-3} = 1.5 * 82.3 * 5.9 \left[\frac{1}{22,787.06 \sqrt{1 + \left(\frac{11}{5.9} \sqrt[3]{\frac{E_p}{22,787.06}} \right)^2}} + \frac{1}{E_p} \left(1 - \frac{1}{\sqrt{1 + \left(\frac{11}{5.9} \right)^2}} \right) \right]$$

E_p solved by HP solver program = 506,094.29 psi

$$a_e = \sqrt{5.9^2 + \left(11 * \sqrt[3]{\frac{506,094.79}{22,787.06}} \right)^2} = 31.48"$$

$$0.7a_e = 22.03"$$

$0.7a_e > r$ therefore, no good

new $r = 22.5"$

$$\frac{24 - 22.5}{24 - 12} = \frac{4.11 - D_r}{4.11 - 5.61} \quad D_r = 4.30 \text{ mils}$$

$$M_r = \frac{9000(1 - 0.5^2)}{3.14 * 22.5 * 4.30 * 10^{-3}} = 22,207.67 \text{ psi}$$

$$6.75 \times 10^{-3} = 1.5 \times 82.3 \times 5.9 \left[\frac{1}{22,207.67 \sqrt{1 + \left(\frac{11}{5.9} \sqrt[3]{\frac{E_p}{22,207.67}} \right)^2}} + \frac{1 - \frac{1}{\sqrt{1 + \left(\frac{11}{5.9} \right)^2}}}{E_p} \right]$$

E_p solved by HP solver program = 526,316.31 psi

$$a_e = \sqrt{5.9^2 + \left(11 \sqrt[3]{\frac{526,316.31}{22,207.67}} \right)^2} = 32.14"$$

$$0.7a_e = 22.50$$

$$0.7a_e < r \quad OK$$

Therefore, for MT1 on 2/11/92: $M_r = 22,207.67$ psi

$$E_p = 526,316.31$$
 psi

Two layers system - AC and subgrade

Est. mid-depth tem. (deg. F)	Man. Bkcd. Ep (ksi)	MODULUS Ep (ksi)	Man. Bkcd. Mr (ksi)	MODULUS Mr (ksi)
35.6	415.6	757	22.29	17.70
38.7	524.5	797	22.26	16.80
42.1	512.44	754	22.66	17.30
46.1	303.02	574	20.50	15.70
49.8	533.54	792	26.17	17.10
50	397.12	566	20.18	16.50
50.4	254.87	449	16.81	13.10
50.8	591.62	899	31.03	19.60
58.8	324.13	511	19.89	16.30
64.6	187.3	319	14.91	12.00
71.9	240.87	327	21.84	14.80
73.3	304.71	411	21.20	13.40
74.8	343.81	487	22.24	17.90
77.2	262.95	346	25.10	17.80
78.2	311.17	432	20.65	16.80
79.2	229.26	308	22.67	15.30
79.4	173.42	265	16.69	13.50
83.6	231.47	314	19.68	12.80
85.1	174.56	246	13.47	8.80
87.8	290.9	393	21.63	18.00
90	216.36	290	20.34	17.20
95.3	65.21	104	10.06	6.80
95.8	80.74	130	12.10	8.60
97.3	95.86	128	13.23	8.10
98.4	78.78	129	10.78	7.70
103.5	129.33	183	18.69	14.40
108.6	61.45	82	11.50	6.30

Comparison of Modulus E_{ac} vs. Asphalt Institute (AI) Regression Values for E_{ac}

tp (deg. F)	AI E _{ac} (ksi)	Mod. E _{ac} (ksi)
35.6	2047.94	1497
38.7	1908.83	2130
42.1	1756.14	2011
46.1	1578.92	1069
49.8	1419.67	1676
50.0	1411.23	1526
50.4	1394.40	883
50.8	1377.66	1808
58.8	1062.13	997
64.6	860.55	615
71.9	643.39	613
73.3	606.50	815
74.8	568.64	1157
77.2	511.65	657
78.2	489.17	1097
79.2	467.44	589
79.4	463.18	504
83.6	380.29	642
85.1	353.62	495
87.8	309.32	974
90.0	276.56	691
95.3	208.98	167
95.8	203.38	215
97.3	187.29	219
98.4	176.18	224
103.5	131.58	364
108.6	96.94	127

SHRP Regional Information Management System Laboratory Data

Temperature (° F)	MT1 E _{ac} average (ksi)	MT2 E _{ac} average (ksi)
41	813.3	739.0
77	509.6	501.6
104	237.0	203.9

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